# STONEFIELD

# STORMWATER MANAGEMENT REPORT ENCLAVE AT MILLINGTON

PROPOSED MIXED-USE RESIDENTIAL & COMMERCIAL DEVELOPMENT BLOCK 12301, LOT 1 & BLOCK 10100, LOT 7.01 50 DIVISION AVENUE MILLINGTON, TOWNSHIP OF LONG HILL MORRIS COUNTY, NEW JERSEY

> PREPARED FOR: PRISM MILLINGTON, LLC

PREPARED BY: STONEFIELD ENGINEERING & DESIGN, LLC 92 PARK AVENUE RUTHERFORD, NEW JERSEY

> REPORT DATE: OCTOBER 25, 2019

CHUCK D. OLIVO, PE, PP, PTOE NJ PE LICENSE #46719

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#### **I.0 PROJECT DESCRIPTION**

Prism Millington, LLC is proposing to redevelop Block 12301, Lot I and Block 10100, Lot 7.01 (herein referred to as the "project site") to accommodate fourteen multi-family residential buildings (140 units total), one commercial building (approximately 4,992 SF), and multiple amenity areas for residents (consisting of a multiple patios throughout the project site, community building and a pool). Additional improvements include multiple patio areas located throughout the project site, off-street parking lots, lighting, and landscaping. The subject property is located within the Township of Long Hill and is bounded by an NJ Transit Railroad to the north, Division Avenue to the east, Stone House Road to the south, and the Passaic River to the west.

Refer to **APPENDIX A** for project maps of the subject site.

# The total project area is 518,332 SF (11.90 acres), the impervious surfaces has been reduced by 101,084 SF (2.32 acres), and the total area of disturbance is 339,160 SF (7.79 acres).

This Stormwater Management Report has been prepared to analyze the potential stormwater runoff impacts of the proposed project and discuss the measures proposed to conform to the stormwater management requirements set forth by the Township of Long Hill, Morris County Soil Conservation District, and the New Jersey Department of Environmental Protection (NJDEP).

#### 2.0 EXISTING CONDITIONS

#### **EXISTING SITE DEVELOPMENT**

The project site is currently occupied by multiple industrial tenants. Under existing conditions, the site contains two one-story industrial buildings, one two-story industrial building, and one three-story industrial building. Access to the site is provided via two full-movement driveways along River Road and a parking lot that opens directly to the road on Stone House Road. There are single family residences to the west and south-west of the site, commercial businesses to the south and east. To the north, there are commercial businesses and a train station. Train tracks run adjacent to the northern property line while the Passaic River runs along the western property line. An Aerial Map depicting the existing site conditions can be found in **APPENDIX A**.

#### **EXISTING TOPOGRAPHY**

The high point of the subject site is at the northeast corner of the site abutting River Road and Division Avenue. River Road drains northwest towards current parking areas of adjacent lots, Division Avenue drains south towards Stone House Road and combines with the run-off of Stone House Road ultimately discharging into the Passaic River. On-site topography slopes toward low points within the site, collected by a stormwater system, and discharged to the Passaic River. Grades on the subject site average between 0% and 3.43%. within the areas surrounding the buildings. However, steeper slopes are seen in the northern and southwestern corners of the subject site.

#### **PROJECT SITE SOILS**

Soil mapping was obtained from the National Resource Conservation Service (NRCS) for the project site and immediate area. Generally, the project site is underlain with one major soil group: silt loam which occupies a majority of the site. Overall, the soils drain well, and runoff flows overland directly to the Passaic River. The table below provide a summary of soils for the project site:

Soil Unit Code	Soil Description	Approximate Project Coverage	Drainage Class	Hydrologic Soil Group
PeoC	Penn Channery Silt Loam, 8% to I 5% Slopes	9.5%	Well drained	В
USPENB	Urban Land-Penn Complex, 0% to 8% Slopes	87.5%	Well drained	С
WATER	Passaic River	3.0%	N/A	N/A

#### TABLE I: NRCS SOIL MAPPING RESULTS

\*USPENB does not have a pre-determined hydraulic soil group due to high variability in the historic fill material utilized. As such, these soils are analyzed as HSG B under pre-existing conditions and HSG D under post-development conditions.

Additional information regarding the NRCS soil mapping can be found in **APPENDIX B**.

#### **EXISTING ENVIRONMENTAL INVENTORY**

The project site is bounded by the Passaic River on the west with a NJDEP Restricted Area separating it from the proposed project site. The river is subject to a special flood hazard area as verified and delineated on FEMA Flood Insurance Map Panel # 3403560005B. The limits of these areas are shown on the Critical Area Plan (Sheet C-17) of the Preliminary and Final Major Site Plans prepared by Stonefield in conjunction with this Report.

#### 3.0 **PROPOSED CONDITIONS**

#### **PROPOSED SITE DEVELOPMENT**

Under the proposed development plan, the project area will include a mixed-use family and commercial development. The proposed development includes the construction of fourteen 10-unit multi-family residences, an 1,800 SF community building, a 4,992 SF retail building, and supporting improvements inclusive of parking facilities, landscaping, utilities, site lighting, and stormwater management measures. The eastern portion of the site that is within the limit of disturbance is being collected via the proposed conveyance system on site and either being sent directly to the Passaic River or to the Municipal conveyance system via catch basins and HDPE pipes. Refer to **APPENDIX A** for a half-size Overall Site Plan depicting the proposed project improvements.

#### **PROPOSED TOPOGRAPHY**

Project site topography and drainage patterns will generally remain similar to existing conditions; however, due to the need for more commercially friendly, ADA compliant grades (1.5% to 3%) various retaining walls will be implemented through the project to make up for the change in grades.

#### ANTICIPATED ENVIRONMENTAL INVENTORY IMPACTS

The proposed redevelopment will not disturb land within environmentally regulated areas (flood hazard area, riparian zone, freshwater wetland ditch, and freshwater wetland transition area). As such, permits and approvals will not be sought from the NJDEP to perform work within these areas.

#### 4.0 STORMWATER MANAGEMENT METHODOLOGY & PARAMETERS

#### HYDROLOGIC METHODOLOGY

The analysis program "HydroCAD" Version 10.0 by HydroCAD Software Solutions was utilized to calculate and plot the runoff hydrographs. The program incorporates the time of concentration, C values, rainfall data, and project drainage areas to calculate the runoff characteristics. The existing and proposed drainage areas have been analyzed utilizing Intensity-Duration-Frequency data was obtained from NOAA for the project area; specifics of the rainfall distribution can be found in Appendix C. Additional key variables utilized in the analysis include:

Variable	Input	Input Variable	
Runoff Calculation Method	SCS TR-20	NRCS Rainfall Frequency Data Set	Morris
Pervious/Impervious CN Calculations	Separate	Storm Intervals (Year Events)	2, 10, 25, 100
Stage-Storage Relationship	Dynamic	Storm Duration	24 Hours
Minimum time of concentration	10 minutes	Storm Curve	NOAA D

#### TABLE 2: HYDROCAD DESIGN VARIABLES

Additional information regarding the hydrologic calculations can be found in **APPENIDX C**.

#### HYDRAULIC METHODOLOGY

The analysis program "HydraFlow Storm Sewers" Version 2018 by Autodesk was utilized to generate hydraulic grade lines through the proposed conveyance system model based on various pipe / junction losses and the runoff tributary to each inlet or discharge structure. Additional key variables utilized in the analysis include:

#### **TABLE 3: HYDRAFLOW DESIGN VARIABLES**

Variable	Input	Variable	Input
Runoff Calculation Method	Rational	Pipe Conveyance Method	Std. Step
C-value for impervious surfaces	0.95	Initial Hydraulic Grade Line	Normalized
C-value for pervious surfaces	0.60	Inlet Drainage Area Delineation	Surveyed
Minimum time of concentration	10 minutes	Inlet Geometry & Capacity	NJDOT Std.

Additional information regarding the hydrologic calculations can be found in **APPENDIX C**.

#### 5.0 STORMWATER ANALYSIS

#### **EXISTING DRAINAGE AREAS**

Under existing conditions, the site is comprised of four drainage areas, and one Point of Interest (POI). The Point of Interest (POI-1) discharges to the Passaic River along the western property line. The site slopes from the northeastern corner of the site to the southwestern portion of the site. The existing slopes are not steep throughout the site with an average grade around 3.5%. See below for a short summary of each area:

Drainage Area	Description	Area Extents	Impervious Area	Time of Concentration
E-IA	Existing Drainage to Existing Conveyance System (18" Pipe)	52,576	9,429	10*
E-IB	Existing Drainage to Existing Conveyance System (42" Pipe)	141,668	141,668	10*
E-IC	Existing Drainage to Existing Conveyance System (15" Pipe)	80,227	78,544	10*
E-ID	Existing Drainage to Municipal System	52,934	43,589	10*
POI (E-1)	Ultimate Point of Interest: Passaic River	327,405 SF	273,230	N/A

#### **TABLE 4: SUMMARY OF EXISTING DRAINAGE AREAS**

\*The minimum time of concentration was utilized due to the high level of impervious coverage and proximity to the Passaic River.

All existing drainage areas were delineated based on field surveying data. Hydrologic calculations and parameters for each drainage area can be found in **APPENDIX C**; specific drainage area delineations and land cover can be found in **APPENDIX D**.

#### **PROPOSED DRAINAGE AREAS**

Under proposed conditions the site is comprised of one (1) point of interest. POI-1 is comprised of four subareas with all areas ultimately discharging to the Passaic River, consistent with existing drainage patterns. The subareas are either collected via proposed catch basins and sent directly to the Passaic River via existing conveyance pipes on-site or to the Municipal conveyance system within Stone House Road. Drainage area P-1A is discharged to the Passaic River via a connection to an existing catch basin on the northwest corner of the site with an outlet pipe 18" in diameter. Drainage area P-1B is discharged to the Passaic River via a connection to an existing manhole located between Building #2 and Building #3 with an outlet pipe 42" in diameter. Drainage area P-1C is discharged to the Passaic River via a connection to an existing catch basin located north of Building #5 with an outlet pipe 15" in diameter. Drainage area P-1D is collected within the Municipal System in Stone House Road via sheet flow to various existing inlets which eventually discharge into the Passaic River. See below for a short summary of each area:

Drainage Area	Description	Area Extents	Impervious Area	Time of Concentration
P-IA	Proposed Drainage to Passaic River via Proposed Conveyance System (18" Pipe)	50,878	20,909	10*
P-IB	Proposed Drainage to Passaic River via Proposed Conveyance System (42" Pipe)	142,558	103,691	10*
P-IC	Proposed Drainage to Passaic River via Proposed Conveyance System (15" Pipe)	81,290	58,221	10*
P-ID	Proposed Drainage Directly to Municipal System	52,679	23,598	10*
POI (P-I)	Ultimate Point of Interest: Passaic River	327,405 SF	206,525 SF	N/A

#### TABLE 5: SUMMARY OF PROPOSED DRAINAGE AREAS

\*The minimum time of concentration was utilized for all drainage areas due to the high level of impervious coverage / land disturbance and proximity to existing and proposed stormwater pipe conveyance system.

All proposed drainage areas were delineated based on the proposed grading design overlain on field survey data. Hydrologic calculations and parameters for each drainage area can be found in **APPENDIX C**; specific drainage area delineations and land cover can be found in **APPENDIX D**.

#### STORMWATER MANAGEMENT DESIGN PARAMETERS

As the proposed improvement will disturb 7.79 acres of land, the project is defined as a "Major Development" as indicated in Town Ordinances and per NJDEP regulations. The proposed project will meet the stormwater quantity requirements by demonstrating that at no point in time does the post-development hydrograph or run-off volumes exceed the pre-development hydrograph or run-off volumes; as the analysis area is the same and impervious surfaces have been decreased, the post-construction quantities will at no point exceed the pre-development quantities. Groundwater recharge requirements do not apply as the site is located within the State Planning Area PA-1. Additionally, water quality requirements do not apply as the site is proposing to decrease impervious surfaces on-site by 2.32 acres per NJAC Section 7:8-5.5. See below for a summary of each design parameter and compliance requirements:

Design Parameter	Design Target for Compliance
Stormwater Runoff Quantity	Demonstrate through hydrologic and hydraulic analysis that for stormwater leaving the site, post-construction runoff hydrographs for the 2, 10, and 100-year storm events do not exceed, at any point in time, the pre-construction runoff hydrographs for the same storm events.
Groundwater Recharge	The project is <b>exempt</b> from groundwater recharge requirements as the project site is located within State Planning Area PA-1 (Metropolitan).
Water Quality	The project is <b>exempt</b> from water quality requirements as the project site is proposing to decrease impervious surfaces on-site per NJAC Section 7:8-5.5

#### TABLE 6: STORMWATER MANAGEMENT DESIGN TARGET SUMMARY

#### STORMWATER RUNOFF QUANTITY

Runoff is controlled through the implementation of the reduction of impervious area on site. The tables below summarize the various drainage areas in relation to flow rates and runoff volume during regulatory storm events:

Drainage Area	2-Year Flow Rate	10-Year Flow Rate	100-Year Flow Rate
E-IA	3.14 CFS	4.89 CFS	8.29 CFS
E-1B	9.18 CFS	13.91 CFS	22.97 CFS
E-IC	5.16 CFS	7.84 CFS	12.98 CFS
E-ID	3.16 CFS CFS	4.93 CFS	8.35 CFS
POI (E-I)	20.64 CFS	31.57 CFS	52.59 CFS

**TABLE 7: SUMMARY OF EXISTING DRAINAGE AREA FLOW RATES & VOLUMES**

#### **TABLE 8: SUMMARY OF PROPOSED DRAINAGE AREA FLOW RATES & VOLUMES**

Drainage Area	2-Year Flow Rate	10-Year Flow Rate	100-Year Flow Rate
P-IA	2.44 CFS	4.15 CFS	7.50 CFS
P-IB	8.13 CFS	12.89 CFS	22.14 CFS
P-IC	4.61 CFS	7.33 CFS	12.60 CFS
P-ID	2.59 CFS	4.35 CFS	7.81 CFS
POI (P-I)	17.77 CFS	28.71 CFS	50.05 CFS

Under post-development conditions the runoff flow rates and volumes are reduced to the undetained drainage areas including Stone Hill Road (E-ID/P-ID). The diverted runoff from these areas are collected in the on-site

stormwater management system (E-IA/P-IA, E-IB/P-IB, and E-IC/P-IC) and conveyed directly to discharge into the Passaic River. The table below outlines the regulatory compliance parameters for runoff quantity on the project site:

Rainfall Event	Existing Flow Rate	Required % Reduction	Required Flow Rate	Proposed Flow Rate	Proposed % Reduction
2-Year Storm	20.64 CFS	N/A	20.39 CFS	17.77 CFS	13.91%
10-Year Storm	31.57 CFS	N/A	31.25 CFS	28.71 CFS	9.06%
100-Year Storm	52.59 CFS	N/A	52.19 CFS	50.05 CFS	4.83%

TABLE 9: STORMWATER RUNOFF QUANTITY COMPLIANCE SUMMARY AT POINT OF INTEREST (E-I / P-I)

The reduction of impervious area on site decreases the flow rate as to ensure that no adverse impacts are anticipated downstream of the project site. Detailed hydrologic calculations for each drainage area can be found in **APPENDIX C**.

#### **GROUNDWATER RECHARGE**

As indicated in the Township Ordinances and NJAC 7:8-5.4, the project site is exempt from groundwater recharge requirements as the site is located within the Metropolitan Planning Area (PA-I) per the State Plan Policy Map and thus qualifies as an Urban Redevelopment Area (which is exempt from groundwater recharge requirements).

#### STORMWATER PIPE CONVEYANCE SYSTEM

The on-site stormwater conveyance system has been sized for the 25-year storm event and is able to safely convey runoff to the proposed stormwater management facilities without overflow or bypass. Detailed hydraulic calculations for the conveyance system can be found in **APPENDIX C**. See below for a table summarizing the various drainage areas during the 25-year storm event:

Tributary Area	Existing Flow Rate	Proposed Flow Rate	Flow Rate Difference	Existing Volume	Proposed Volume	Volume Difference
Drainage Direct to Passaic River (E-1A/P-1A, E-1B/P-1B, and E-1C/P-1C)	32.93 CFS	30.74 CFS	-2.19 CFS	137,739 CF	124,277 CF	-13,462 CF
Drainage Direct to Municipal System (E-ID / P-ID)	6.15 CFS	5.58 CFS	-0.57 CFS	25,348 CF	21,909 CF	-3,439 CF
Overall Drainage to Passaic River (E-1/P-1)	39.08 CFS	36.33 CFS	-2.75 CFS	163,087 CF	146,186 CF	-16,901 CF

#### TABLE 10: SUMMARY OF 25-YEAR STORM (FOR DRAINAGE DIRECT TO PUBLIC ROW OR TRIBUTARY)

The runoff flow rates and volumes that directly tributary to the existing Township stormwater pipe conveyance systems within Stone House Road (E-ID/P-ID) are significantly reduced under proposed conditions. As such, no adverse impacts to the adjacent existing stormwater infrastructure is anticipated. Additionally, the volume of stormwater runoff is proposed to decrease overall to the ultimate point of interest (due to the decrease in impervious coverage on-site) the flow rates are significantly reduced and the proposed stormwater management and soil erosion features ensure that runoff entering the Passaic River is safely conveyed so as to not cause any adverse impacts further downstream.

#### 6.0 STORMWATER FACILITY OPERATIONS & MAINTENANCE

A Stormwater Operations & Maintenance Manual will be submitted for approval to the Morris County Soil Conservation District prior to the start construction. Any required easements or covenants associated with the stormwater improvements will be recorded prior to the start of construction.

#### 7.0 EROSION & SEDIMENT CONTROL

A Soil Erosion & Sediment Control Plan has been prepared in accordance with the latest edition of the Standards for Soil Erosion and Sediment Control in New Jersey. Proposed temporary measures during construction include silt fencing, stabilized construction entrances, inlet filters, and cover for soil stabilization. No land disturbance will occur until a permit has been obtained from the Morris County Soil Conservation District.

#### 8.0 CONCLUSIONS

The proposed project complies with all applicable stormwater management regulations and standards. As such, the project is not anticipated to have any adverse impacts or neighboring properties, downstream watercourses, or conveyance systems within the watershed.

#### 9.0 **R**EFERENCES

- New Jersey Administrative Code Title 7, Chapter 8 Stormwater Management, last amended June 20, 2016 <u>https://www.nj.gov/dep/rules/rules/njac7\_8.pdf</u>
- New Jersey Stormwater Best Management Practices Manual, last revised November 2018 <u>https://www.njstormwater.org/bmp\_manual2.htm</u>
- 3. Township of Long Hill Land Use Ordinance, last amended May 31, 2019

https://clerkshq.com/LongHill-nj

# **APPENDIX A PROJECT FIGURES**

### **INVENTORY**

FIGURE I: USGS LOCATION MAP

FIGURE 2: AERIAL MAP

FIGURE 3: TAX & ZONING MAP

FIGURE 4: FEMA MAP

FIGURE 5: SITE PLAN (NOT TO SCALE)













Projec

laps/2019-06-10



1717-17298 PRISM CAPITAL - 50 DIVISION AVENUE, MILLINGTON, NJICADDIPLOT/LDP-04-SITE PLOTTED: 10/29/20

LAND USE AND ZONING LOCK 12301, LOT 1 & BLOCK 10100, LOT 7.01 ROPOSED REDEVELOPMENT (MU-O ZONE)									
							1ITTED USE		
1ITTED USE	-								
NE MU-O QUIREMENTS	EXISTING	PROPOSED							
00 SF	N/A	4,992 SF							
ACRES (495,786.26 SF)	11.90 ACRES (518,322 SF)	11.90 ACRES (518,322 SF)							
8 APARTMENTS APARTMENTS / I ACRE)	N/A	I 40 APARTMENTS (I I.8 APARTMENTS / I ACRE)							
	N/A	15% (21 UNITS)							
FT	532.98 FT	532.98 FT							
STORIES / 35 FT	I STORY / 20 FT	I STORY / < 20 FT							
ORIES / 45 FT	3 STORIES / 45 FT	3 STORIES / < 45 FT							
т	20.9 FT	50.0 FT							
T	20.9 FT	20.0 FT							
T	9.3 FT (EN)	33.8 FT							
T	49.2 FT	30.0 FT							
Т	241.1 FT	90.1 FT							
0% (103,664.4 SF)	29.7% (154,190 SF) (EN)	14.1% (72,882 SF)							
)% (207,323.2 SF)	59.3% (307,609 SF) (EN)	39.8% (206,525 SF)							
) (259,161 SF)	0.47 (244,490 SF)	0.39 (203,194 SF)							
Т	49.2 FT	20.0 FT							

OFF-STREET PARKING REQUIREMENTS					
REQUIRED	PROPOSED				
RETAIL PARKING 4,992 SF x (I SPACE / 200 SF) = 25 SPACES	25 SPACES				
RESIDENTIAL PARKING (GARDEN APARTMENTS): 126 - TWO BR UNIT × (2.0 SPACES / UNIT) = 252 SPACES 14 - THREE BR UNIT × (2.1 SPACES / UNIT) = 29 SPACES TOTAL = 281 SPACES	84 DRIVEWAY SPACES* 84 GARAGE SPACES* 114 GUEST SPACES				
TOTAL REQUIRED PARKING: 306 SPACES	307 SPACES				
REQUIREMENTS FOR ATTACHED UNITS INCLUDE PROVISIONS FOR GUEST PARKING (0.5 SPACES PER DWELLING UNIT). GUEST PARKING MUST EITHER BE PROVIDED FOR ON STREET OR IN COMMON PARKING AREAS. I40 UNITS x (.5 SPACES / UNIT) = 70 GUEST PARKING SPACES	I I 4 SPACES**				
REQUIRED ADA PARKING SPACES 2% OF TOTAL PARKING PROVIDED 306 SPACES × 0.02 = 6.12 ≈ 7 SPACES	10 ADA PARKING SPACES (INCLUSIVE OF 2 VAN ACCESSIBLE AND 2 SPACES WITHIN GARAGES				
PARKING AREA DESIGN: INGRESS AND EGRESS DRIVE WIDTH TWO-WAY = 24 FT	24.0 FT				
PARKING AREA DESIGN: DRIVE AISLE WIDTH 90 DEGREE PARKING = 24 FT	24.0 FT				
OFF STREET PARKING SPACE DIMENSIONS WIDTH = 9 FT LENGTH = 18 FT	WIDTH = 9.0 FT LENGTH = 18.0 FT				
REQUIRED LOADING BERTHS: RETAIL USE - 0 SF TO 5,000 SF: 0 LOADING BERTH	0 LOADING BERTHS				

(\*) PER NJAC 5:21-4.14 (d) 2, ONE-CAR GARAGE AND DRIVEWAY COMBINATION SHALL COUNT AS 2.0 OFF-STREET PARKING SPACES. PROVIDED DRIVEWAY MEASURES A MINIMUM 18FT IN LENGTH.
 (\*\*) ALL SPACES NOT LOCATED WITHIN GARAGES AND DRIVEWAYS ARE CONSIDERED GUEST PARKING

SYMBOL

### DESCRIPTION

PROPERTY LINE

SETBACK LINE

SAWCUT LINE

PROPOSED CURB

PROPOSED DEPRESSED CURB

PROPOSED FLUSH CURB

PROPOSED SIGNS / BOLLARDS

PROPOSED BUILDING

PROPOSED CONCRETE

PROPOSED BUILDING DOORS

PROPOSED RETAINING WALL

### GENERAL NOTES

- I. THE CONTRACTOR SHALL VERIFY AND FAMILIARIZE THEMSELVES WITH THE EXISTING SITE CONDITIONS AND THE PROPOSED SCOPE OF WORK (INCLUDING DIMENSIONS, LAYOUT, ETC.) PRIOR TO INITIATING THE IMPROVEMENTS IDENTIFIED WITHIN THESE DOCUMENTS. SHOULD ANY DISCREPANCY BE FOUND BETWEEN THE EXISTING SITE CONDITIONS AND THE PROPOSED WORK THE CONTRACTOR SHALL NOTIFY STONEFIELD ENGINEERING & DESIGN, LLC. PRIOR TO THE START OF CONSTRUCTION.
- 2. THE CONTRACTOR SHALL OBTAIN ALL NECESSARY PERMITS AND ENSURE THAT ALL REQUIRED APPROVALS HAVE BEEN OBTAINED PRIOR TO THE START OF CONSTRUCTION. COPIES OF ALL REQUIRED PERMITS AND APPROVALS SHALL BE KEPT ON SITE AT ALL TIMES DURING CONSTRUCTION.
- 3. ALL CONTRACTORS WILL, TO THE FULLEST EXTENT PERMITTED BY LAW, INDEMNIFY AND HOLD HARMLESS STONEFIELD ENGINEERING & DESIGN, LLC. AND IT'S SUB-CONSULTANTS FROM AND AGAINST ANY DAMAGES AND LIABILITIES INCLUDING ATTORNEY'S FEES ARISING OUT OF CLAIMS BY EMPLOYEES OF THE CONTRACTOR IN ADDITION TO CLAIMS CONNECTED TO THE PROJECT AS A RESULT OF NOT CARRYING THE PROPER INSURANCE FOR WORKERS COMPENSATION, LIABILITY INSURANCE, AND LIMITS OF COMMERCIAL GENERAL
- LIABILITY INSURANCE.
  4. THE CONTRACTOR SHALL NOT DEVIATE FROM THE PROPOSED IMPROVEMENTS IDENTIFIED WITHIN THIS PLAN SET UNLESS APPROVAL IS PROVIDED IN WRITING BY STONEFIELD ENGINEERING & DESIGN, LLC.
- THE CONTRACTOR IS RESPONSIBLE TO DETERMINE THE MEANS AND METHODS OF CONSTRUCTION.
   THE CONTRACTOR SHALL NOT PERFORM ANY WORK OR CAUSE DISTURBANCE ON A PRIVATE PROPERTY NOT CONTROLLED BY THE PERSON OR ENTITY WHO HAS AUTHORIZED THE WORK WITHOUT PRIOR WRITTEN CONSENT FROM THE OWNER OF THE PRIVATE
- PROPERTY.
  7. THE CONTRACTOR IS RESPONSIBLE TO RESTORE ANY DAMAGED OR UNDERMINED STRUCTURE OR SITE FEATURE THAT IS IDENTIFIED TO REMAIN ON THE PLAN SET. ALL REPAIRS SHALL USE NEW MATERIALS TO RESTORE THE FEATURE TO ITS EXISTING CONDITION AT THE CONTRACTORS EXPENSE.
- CONTRACTOR IS RESPONSIBLE TO PROVIDE THE APPROPRIATE SHOP DRAWINGS, PRODUCT DATA, AND OTHER REQUIRED SUBMITTALS FOR REVIEW. STONEFIELD ENGINEERING & DESIGN, LLC. WILL REVIEW THE SUBMITTALS IN ACCORDANCE WITH THE DESIGN INTENT AS REFLECTED WITHIN THE PLAN SET.
   THE CONTRACTOR IS RESPONSIBLE FOR TRAFFIC CONTROL IN
- ACCORDANCE WITH MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, LATEST EDITION. 10. THE CONTRACTOR IS REQUIRED TO PERFORM ALL WORK IN THE PUBLIC RIGHT-OF-WAY IN ACCORDANCE WITH THE APPROPRIATE GOVERNING AUTHORITY AND SHALL BE RESPONSIBLE FOR THE
- PROCUREMENT OF STREET OPENING PERMITS.
  11. THE CONTRACTOR IS REQUIRED TO RETAIN AN OSHA CERTIFIED SAFETY INSPECTOR TO BE PRESENT ON SITE AT ALL TIMES DURING CONSTRUCTION & DEMOLITION ACTIVITIES.
  12. SHOULD AN EMPLOYEE OF STONEFIELD ENGINEERING & DESIGN, LLC.
- BE PRESENT ON SITE AT ANY TIME DURING CONSTRUCTION, IT DOES NOT RELIEVE THE CONTRACTOR OF ANY OF THE RESPONSIBILITIES AND REQUIREMENTS LISTED IN THE NOTES WITHIN THIS PLAN SET.

40' 0' 40' 80' GRAPHIC SCALE IN FEET I'' = 40'



# APPENDIX B NRCS Soils Report





United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

## Custom Soil Resource Report for Morris County, New Jersey





MAP L	EGEND	MAP INFORMATION		
Area of Interest (AOI) Area of Interest (AOI)	<ul><li>Spoil Area</li><li>Stony Spot</li></ul>	The soil surveys that comprise your AOI were mapped at 1:24,000.		
Soil Map Unit Polygons Soil Map Unit Lines Soil Map Unit Lines Soil Map Unit Lines Soli Map Unit Points Borrow Pit Soli Map Unit Points Clay Spot Clay Spot Gravel Pit Gravelly Spot	<ul> <li>Very Stony Spot</li> <li>Wet Spot</li> <li>Other</li> <li>Special Line Features</li> <li>Water Features</li> <li>Streams and Canals</li> <li>Transportation</li> <li>Rails</li> <li>Interstate Highways</li> <li>US Routes</li> </ul>	<ul> <li>Warning: Soil Map may not be valid at this scale.</li> <li>Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.</li> <li>Please rely on the bar scale on each map sheet for map measurements.</li> <li>Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)</li> </ul>		
<ul> <li>Landfill</li> <li>Lava Flow</li> <li>Marsh or swamp</li> <li>Mine or Quarry</li> <li>Miscellaneous Water</li> <li>Perennial Water</li> <li>Rock Outcrop</li> <li>Saline Spot</li> <li>Sandy Spot</li> <li>Severely Eroded Spot</li> <li>Sinkhole</li> <li>Slide or Slip</li> <li>Sodic Spot</li> </ul>	Local Roads	<ul> <li>Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.</li> <li>This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.</li> <li>Soil Survey Area: Morris County, New Jersey Survey Area Data: Version 13, Sep 13, 2018</li> <li>Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.</li> <li>Date(s) aerial images were photographed: Dec 31, 2009—Feb 26, 2017</li> <li>The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background</li> </ul>		

### **Map Unit Legend**

		-	
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
PeoC	Penn channery silt loam, 8 to 15 percent slopes	1.1	9.3%
USPENB	Urban land-Penn complex, 0 to 8 percent slopes	10.7	88.7%
WATER	Water	0.2	2.0%
Totals for Area of Interest		12.1	100.0%

### Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The

delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

#### Morris County, New Jersey

#### PeoC—Penn channery silt loam, 8 to 15 percent slopes

#### **Map Unit Setting**

National map unit symbol: 2tt83 Elevation: 250 to 800 feet Mean annual precipitation: 38 to 53 inches Mean annual air temperature: 43 to 57 degrees F Frost-free period: 170 to 240 days Farmland classification: Farmland of statewide importance

#### **Map Unit Composition**

Penn and similar soils: 80 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Penn**

#### Setting

Landform: Hills Landform position (two-dimensional): Shoulder, backslope, summit Landform position (three-dimensional): Interfluve, side slope Down-slope shape: Convex Across-slope shape: Linear Parent material: Residuum weathered from shale and siltstone

#### **Typical profile**

Ap - 0 to 10 inches: channery silt loam Bt1 - 10 to 15 inches: channery silt loam Bt2 - 15 to 19 inches: channery silt loam Bt3 - 19 to 22 inches: channery loam C - 22 to 28 inches: very channery loam R - 28 to 38 inches: bedrock

#### **Properties and qualities**

Slope: 8 to 15 percent
Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.14 to 1.28 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 4.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3e Hydrologic Soil Group: B Hydric soil rating: No

#### **Minor Components**

#### Klinesville

Percent of map unit: 10 percent Landform: Hills Landform position (two-dimensional): Backslope, shoulder Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

#### Croton

Percent of map unit: 5 percent Landform: Depressions Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope Down-slope shape: Linear, concave Across-slope shape: Linear, concave Hydric soil rating: Yes

#### Readington

Percent of map unit: 5 percent Landform: Depressions Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Linear, concave Across-slope shape: Linear, concave Hydric soil rating: No

#### USPENB—Urban land-Penn complex, 0 to 8 percent slopes

#### Map Unit Setting

National map unit symbol: 13q0b Elevation: 250 to 1,300 feet Mean annual precipitation: 30 to 64 inches Mean annual air temperature: 46 to 79 degrees F Frost-free period: 131 to 178 days Farmland classification: Not prime farmland

#### Map Unit Composition

Urban land: 55 percent Penn and similar soils: 35 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Urban Land**

#### Setting

Landform: Hills Down-slope shape: Linear, convex Across-slope shape: Linear

*Parent material:* Surface covered by pavement, concrete, buildings, and other structures underlain by disturbed and natural soil material

#### **Typical profile**

C - 0 to 60 inches: variable

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8s Hydric soil rating: Unranked

#### **Description of Penn**

#### Setting

Landform: Hills Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Convex Parent material: Fine-loamy residuum weathered from acid reddish shale, siltstone, and fine-grain sandstone

#### **Typical profile**

A - 0 to 8 inches: channery silt loam
BA - 8 to 14 inches: channery silt loam
B - 14 to 24 inches: channery silt loam
BC - 24 to 30 inches: channery silt loam
C - 30 to 36 inches: very channery silt loam
R - 36 to 80 inches: weathered bedrock

#### **Properties and qualities**

Slope: 0 to 6 percent
Depth to restrictive feature: 20 to 39 inches to lithic bedrock
Natural drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 5.8 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: C Hydric soil rating: No

#### Minor Components

#### Klinesville

Percent of map unit: 5 percent Landform: Hills Landform position (two-dimensional): Shoulder Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: No

#### Reaville

Percent of map unit: 5 percent Landform: Interfluves Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

#### WATER—Water

#### Map Unit Setting

National map unit symbol: b0p9 Mean annual precipitation: 30 to 64 inches Mean annual air temperature: 46 to 79 degrees F Frost-free period: 131 to 178 days Farmland classification: Not prime farmland

#### Map Unit Composition

*Water:* 100 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

# APPENDIX C Hydrologic & Hydraulic Calculations

### **INVENTORY**

- C-I: NOAA RAINFALL FREQUENCY DATA
- C-2: HYDROCAD NODE SCHEMATIC DIAGRAM
- C-3: HYDROCAD HYDROLOGIC CALCULATIONS
- C-4: HYDRAFLOW PIPE NETWORK SUMMARY CHART
- C-5: HYDRAFLOW HYDRAULIC PIPE ANALYSIS



Precipitation Frequency Data Server



NOAA Atlas 14, Volume 2, Version 3 Location name: Millington, New Jersey, USA\* Latitude: 40.6718°, Longitude: -74.5245° Elevation: 249.16 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

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PF\_tabular | PF\_graphical | Maps\_&\_aerials

#### PF tabular

PD	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup>									
Duration	Average rec				ge recurren	nce interval (years)				
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.335</b>	<b>0.399</b>	<b>0.471</b>	<b>0.524</b>	<b>0.588</b>	<b>0.632</b>	<b>0.676</b>	<b>0.715</b>	<b>0.764</b>	<b>0.799</b>
	(0.305-0.369)	(0.363-0.439)	(0.428-0.519)	(0.475-0.577)	(0.531-0.646)	(0.569-0.695)	(0.605-0.743)	(0.636-0.785)	(0.674-0.840)	(0.700-0.881)
10-min	<b>0.534</b>	<b>0.637</b>	<b>0.754</b>	<b>0.837</b>	<b>0.936</b>	<b>1.01</b>	<b>1.07</b>	<b>1.13</b>	<b>1.21</b>	<b>1.25</b>
	(0.486-0.589)	(0.580-0.702)	(0.685-0.830)	(0.759-0.921)	(0.845-1.03)	(0.904-1.11)	(0.960-1.18)	(1.01-1.24)	(1.06-1.33)	(1.10-1.38)
15-min	<b>0.667</b>	<b>0.800</b>	<b>0.953</b>	<b>1.06</b>	<b>1.19</b>	<b>1.27</b>	<b>1.36</b>	<b>1.43</b>	<b>1.52</b>	<b>1.58</b>
	(0.608-0.735)	(0.729-0.881)	(0.866-1.05)	(0.959-1.16)	(1.07-1.30)	(1.15-1.40)	(1.21-1.49)	(1.27-1.57)	(1.34-1.67)	(1.38-1.74)
30-min	<b>0.913</b>	<b>1.10</b>	<b>1.35</b>	<b>1.53</b>	<b>1.75</b>	<b>1.91</b>	<b>2.07</b>	<b>2.22</b>	<b>2.41</b>	<b>2.54</b>
	(0.832-1.01)	(1.00-1.22)	(1.23-1.49)	(1.39-1.68)	(1.58-1.93)	(1.72-2.10)	(1.85-2.28)	(1.97-2.44)	(2.13-2.65)	(2.23-2.81)
60-min	<b>1.14</b>	<b>1.38</b>	<b>1.73</b>	<b>1.99</b>	<b>2.33</b>	<b>2.59</b>	<b>2.85</b>	<b>3.11</b>	<b>3.46</b>	<b>3.71</b>
	(1.04-1.25)	(1.26-1.53)	(1.58-1.91)	(1.81-2.19)	(2.11-2.56)	(2.33-2.85)	(2.55-3.13)	(2.77-3.42)	(3.05-3.80)	(3.26-4.10)
2-hr	<b>1.39</b>	<b>1.70</b>	<b>2.15</b>	<b>2.50</b>	<b>2.99</b>	<b>3.39</b>	<b>3.80</b>	<b>4.23</b>	<b>4.83</b>	<b>5.30</b>
	(1.26-1.54)	(1.54-1.88)	(1.95-2.38)	(2.26-2.77)	(2.68-3.30)	(3.02-3.74)	(3.37-4.19)	(3.72-4.66)	(4.20-5.34)	(4.57-5.87)
3-hr	<b>1.56</b>	<b>1.90</b>	<b>2.41</b>	<b>2.81</b>	<b>3.36</b>	<b>3.80</b>	<b>4.26</b>	<b>4.74</b>	<b>5.41</b>	<b>5.94</b>
	(1.41-1.73)	(1.72-2.11)	(2.18-2.68)	(2.53-3.11)	(3.01-3.71)	(3.40-4.20)	(3.78-4.71)	(4.18-5.24)	(4.71-5.99)	(5.12-6.59)
6-hr	<b>2.00</b>	<b>2.43</b>	<b>3.08</b>	<b>3.61</b>	<b>4.37</b>	<b>5.00</b>	<b>5.67</b>	<b>6.40</b>	<b>7.44</b>	<b>8.30</b>
	(1.82-2.23)	(2.21-2.70)	(2.79-3.41)	(3.26-3.98)	(3.90-4.80)	(4.44-5.49)	(4.99-6.23)	(5.58-7.01)	(6.39-8.17)	(7.05-9.13)
12-hr	<b>2.49</b> (2.26-2.77)	<b>3.02</b> (2.74-3.36)	<b>3.85</b> (3.48-4.27)	<b>4.55</b> (4.10-5.02)	<b>5.58</b> (4.98-6.13)	<b>6.46</b> (5.72-7.09)	<b>7.43</b> (6.50-8.14)	<b>8.49</b> (7.34-9.31)	<b>10.1</b> (8.55-11.0)	<b>11.4</b> (9.55-12.5)
24-hr	<b>2.83</b>	<b>3.42</b>	<b>4.36</b>	<b>5.15</b>	<b>6.34</b>	<b>7.36</b>	<b>8.48</b>	<b>9.73</b>	<b>11.6</b>	<b>13.2</b>
	(2.62-3.08)	(3.17-3.72)	(4.03-4.74)	(4.75-5.60)	(5.81-6.87)	(6.69-7.97)	(7.65-9.19)	(8.68-10.6)	(10.2-12.6)	(11.4-14.4)
2-day	<b>3.32</b>	<b>4.02</b>	<b>5.11</b>	<b>6.01</b>	<b>7.33</b>	<b>8.44</b>	<b>9.64</b>	<b>10.9</b>	<b>12.9</b>	<b>14.4</b>
	(3.06-3.64)	(3.70-4.40)	(4.69-5.59)	(5.51-6.57)	(6.67-8.00)	(7.64-9.20)	(8.66-10.5)	(9.74-12.0)	(11.3-14.1)	(12.5-15.9)
3-day	<b>3.50</b>	<b>4.23</b>	<b>5.36</b>	<b>6.28</b>	<b>7.63</b>	<b>8.75</b>	<b>9.95</b>	<b>11.3</b>	<b>13.1</b>	<b>14.7</b>
	(3.23-3.82)	(3.90-4.61)	(4.93-5.84)	(5.77-6.84)	(6.96-8.30)	(7.95-9.52)	(8.98-10.8)	(10.1-12.3)	(11.6-14.4)	(12.8-16.2)
4-day	<b>3.68</b>	<b>4.44</b>	<b>5.61</b>	<b>6.56</b>	<b>7.93</b>	<b>9.06</b>	<b>10.3</b>	<b>11.6</b>	<b>13.4</b>	<b>15.0</b>
	(3.40-4.00)	(4.11-4.83)	(5.17-6.10)	(6.03-7.12)	(7.26-8.60)	(8.25-9.84)	(9.29-11.2)	(10.4-12.6)	(11.9-14.7)	(13.1-16.5)
7-day	<b>4.34</b> (4.03-4.70)	<b>5.21</b> (4.84-5.64)	<b>6.46</b> (5.99-6.99)	<b>7.49</b> (6.92-8.10)	<b>8.96</b> (8.24-9.69)	<b>10.2</b> (9.31-11.0)	<b>11.5</b> (10.4-12.4)	<b>12.9</b> (11.6-14.0)	<b>14.8</b> (13.2-16.2)	<b>16.5</b> (14.5-18.0)
10-day	<b>4.98</b>	<b>5.95</b>	<b>7.27</b>	<b>8.34</b>	<b>9.86</b>	<b>11.1</b>	<b>12.4</b>	<b>13.7</b>	<b>15.7</b>	<b>17.2</b>
	(4.64-5.36)	(5.55-6.40)	(6.76-7.82)	(7.75-8.98)	(9.11-10.6)	(10.2-11.9)	(11.3-13.4)	(12.5-14.9)	(14.0-17.0)	(15.3-18.7)
20-day	<b>6.72</b> (6.32-7.17)	<b>7.97</b> (7.49-8.51)	<b>9.51</b> (8.93-10.1)	<b>10.7</b> (10.1-11.4)	<b>12.3</b> (11.5-13.2)	<b>13.6</b> (12.7-14.5)	<b>14.9</b> (13.8-15.9)	<b>16.2</b> (15.0-17.4)	<b>17.9</b> (16.4-19.3)	<b>19.3</b> (17.5-20.8)
30-day	<b>8.37</b>	<b>9.89</b>	<b>11.6</b>	<b>12.8</b>	<b>14.5</b>	<b>15.7</b>	<b>17.0</b>	<b>18.2</b>	<b>19.7</b>	<b>20.9</b>
	(7.92-8.85)	(9.35-10.4)	(10.9-12.2)	(12.1-13.5)	(13.6-15.3)	(14.8-16.6)	(15.9-17.9)	(16.9-19.3)	(18.3-21.0)	(19.2-22.2)
45-day	<b>10.6</b> (10.1-11.2)	<b>12.5</b> (11.9-13.2)	<b>14.4</b> (13.7-15.2)	<b>15.8</b> (15.0-16.7)	<b>17.7</b> (16.7-18.6)	<b>19.1</b> (18.0-20.1)	<b>20.4</b> (19.2-21.5)	<b>21.6</b> (20.3-22.8)	<b>23.2</b> (21.7-24.6)	<b>24.3</b> (22.6-25.8)
60-day	<b>12.8</b> (12.1-13.4)	<b>15.0</b> (14.2-15.7)	<b>17.1</b> (16.3-17.9)	<b>18.7</b> (17.8-19.6)	<b>20.6</b> (19.6-21.7)	<b>22.1</b> (20.9-23.2)	<b>23.4</b> (22.2-24.7)	<b>24.7</b> (23.3-26.0)	<b>26.2</b> (24.6-27.7)	<b>27.2</b> (25.6-28.9)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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#### **PF** graphical



2019-10-28_Stormwater Calculations Prepared by Microsoft HydroCAD® 10.00-22 s/n 06682 © 2018 HydroCAD Software	NOAA 24-hr D 2-Year Rainfall=3.42" Printed 10/30/2019 Solutions LLC Page 1
Time span=0.00-30.00 hrs, dt= Runoff by SCS TR-20 method, UH=5 Reach routing by Dyn-Stor-Ind method - Po	0.01 hrs, 3001 points SCS, Split Pervious/Imperv. and routing by Dyn-Stor-Ind method
Subcatchment E-1A: Existing Drainage to Runoff Area	=52,576 sf 82.07% Impervious Runoff Depth=2.90" Tc=10.0 min CN=80/98 Runoff=3.14 cfs 12,694 cf
Subcatchment E-1B: Existing Drainage Runoff Area=14	41,668 sf 100.00% Impervious Runoff Depth=3.19" Tc=10.0 min CN=0/98 Runoff=9.18 cfs 37,621 cf
Subcatchment E-1C: Existing Drainageto Runoff Area	=80,227 sf 97.90% Impervious Runoff Depth=3.16" Tc=10.0 min CN=83/98 Runoff=5.16 cfs 21,109 cf
Subcatchment E-1D: Existing Drainage to Runoff Area	=52,934 sf 82.35% Impervious Runoff Depth=2.90" Tc=10.0 min CN=80/98 Runoff=3.16 cfs 12,801 cf
Subcatchment P-1A: Proposed Drainage to Runoff Area	=50,878 sf 41.10% Impervious Runoff Depth=2.24" Tc=10.0 min CN=80/98 Runoff=2.44 cfs 9,481 cf
Subcatchment P-1B: Proposed Drainage Runoff Area=	142,558 sf 72.74% Impervious Runoff Depth=2.75" Tc=10.0 min CN=80/98 Runoff=8.13 cfs 32,631 cf
Subcatchment P-1C: Proposed Drainage to Runoff Area	=81,290 sf 71.62% Impervious Runoff Depth=2.73" Tc=10.0 min CN=80/98 Runoff=4.61 cfs 18,485 cf
Subcatchment P-1D: Proposed Drainage to Runoff Area	=52,679 sf 44.80% Impervious Runoff Depth=2.30" Tc=10.0 min CN=80/98 Runoff=2.59 cfs 10,079 cf
Link E-1: Existing Drainage to Passaic River	Inflow=20.64 cfs 84,226 cf Primary=20.64 cfs 84,226 cf
Link P-1: Proposed Drainage to Passaic River	Inflow=17.77 cfs 70,677 cf Primary=17.77 cfs 70,677 cf

Total Runoff Area = 654,810 sf Runoff Volume = 154,903 cf Average Runoff Depth = 2.84" 21.60% Pervious = 141,443 sf 78.40% Impervious = 513,367 sf

#### Summary for Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)

Runoff = 3.14 cfs @ 12.17 hrs, Volume= 12,694 cf, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

	Area (sf)	CN	Description				
*	43,147	98	Impervious Areas				
	9,429	80	>75% Grass cover, Good, HSG D				
	52,576 95 Weighted Average			verage			
	9,429	80	17.93% Pervious Area				
	43,147	98	82.07% Imp	pervious Ar	rea		
_				_			
-	Tc Length	Slop	e Velocity	Capacity	Description		
(mi	n) (feet)	(ft/f	t) (ft/sec)	(cfs)			
10	.0				Direct Entry, Direct		

#### Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)



#### Summary for Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)

Runoff = 9.18 cfs @ 12.17 hrs, Volume= 37,621 cf, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

	A	rea (sf)	CN [	Description				
*	1	41,668	98 I	Impervious Areas				
	1	41,668	98 ´	100.00% Im	npervious A	Area		
	Тс	Length	Slope	Velocity	Capacity	Description		
(	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	- -		
	10.0					Direct Entry, Direct		

Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)



#### Summary for Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)

Page 4

Runoff 5.16 cfs @ 12.17 hrs, Volume= 21,109 cf, Depth= 3.16" =

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

1	10.0				Direct Entry, Direct			
(r	min) (feet)	(ft/	ft) (ft/sec)	(cfs)				
	re Lengin	Sioh	be velocity	Capacity	Description			
	To Longth	Clor		Conocity	Description			
	78,544	98	97.90% lmp	97.90% Impervious Area				
	1,003	03	2.10% Perv	2.10% Pervious Area				
	1 602	02	2 100/ Don					
	80.227	98	Weighted A	Veighted Average				
	1,330	80	>75% Gras	>75% Grass cover, Good, HSG D				
	353	96	Gravel surfa	Gravel surface, HSG D				
*	78,544	98	Impervious	npervious Areas				
	Area (st)	CN	Description					
	Area (cf)	CN	Description					

Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)


#### Summary for Subcatchment E-1D: Existing Drainage to Municipal System

Runoff 3.16 cfs @ 12.17 hrs, Volume= 12,801 cf, Depth= 2.90" =

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

	Area (sf)	CN	Description						
*	43,589	98	Impervious /	Areas					
	9,345	80	>75% Grass	75% Grass cover, Good, HSG D					
	52,934	95	Weighted Av	verage					
	9,345	80	17.65% Per	17.65% Pervious Area					
	43,589	98	82.35% Imp	ervious Are	ea				
,	Tc Length	Slop	e Velocity	Capacity	Description				
(I	min) (feet)	(ft/	t) (ft/sec)	(CTS)					
	10.0				Direct Entry, Direct				

#### Subcatchment E-1D: Existing Drainage to Municipal System



## Summary for Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)

Runoff = 2.44 cfs @ 12.17 hrs, Volume= 9,481 cf, Depth= 2.24"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

	Area (sf)	CN	Description					
*	20,909	98	Impervious	Areas				
	29,969	80	>75% Grass	s cover, Go	bod, HSG D			
	50,878	87	Weighted A	verage				
	29,969	80	58.90% Per	58.90% Pervious Area				
	20,909	98	41.10% Imp	pervious Are	ea			
	Tc Length	Slop	e Velocity	Capacity	Description			
(n	nin) (feet)	(ft/1	t) (ft/sec)	(cfs)				
1	0.0				Direct Entry, Direct			

#### Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)



## Summary for Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)

Runoff = 8.13 cfs @ 12.17 hrs, Volume= 32,631 cf, Depth= 2.75"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

	Area (sf)	CN	Description					
*	103,691	98	Impervious	Areas				
	38,867	80	>75% Gras	s cover, Go	ood, HSG D			
	142,558	93	Weighted Average					
	38,867	367 80 27.26% Pervious Area						
	103,691	98	72.74% lmp	pervious Ar	ea			
-				0	Description			
í í	c Length	Slop	e Velocity	Capacity	Description			
(mi	n) (feet)	(ft/f	t) (ft/sec)	(cts)				
10	0				Direct Entry, Direct			

#### Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)

Runoff = 4.61 cfs @ 12.17 hrs, Volume= 18,485 cf, Depth= 2.73"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

	Area (sf)	CN	Description						
*	58,221	98	Impervious	Areas					
	23,069	80	>75% Gras	s cover, Go	ood, HSG D				
	81,290	93	Weighted A	verage					
	23,069	80	28.38% Per	28.38% Pervious Area					
	58,221	98	71.62% Imp	pervious Ar	rea				
_				_					
	Fc Length	Slop	e Velocity	Capacity	Description				
(mi	n) (feet)	(ft/f	t) (ft/sec)	(cfs)					
10	.0				Direct Entry, Direct				

#### Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)



#### Summary for Subcatchment P-1D: Proposed Drainage to Municipal System

Runoff 2.59 cfs @ 12.17 hrs, Volume= 10,079 cf, Depth= 2.30" =

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

	Area (sf)	CN	Description						
*	23,598	98	Impervious	Areas					
	29,081	80	>75% Gras	s cover, Go	bod, HSG D				
	52,679	88	Weighted A	Veighted Average					
	29,081	80	55.20% Per	55.20% Pervious Area					
	23,598	98	44.80% Imp	ervious Ar	ea				
	Tc Length	Slop	e Velocity	Capacity	Description				
(r	nin) (feet)	(ft/f	t) (ft/sec)	(cfs)					
1	0.0				Direct Entry, Direct				

### Subcatchment P-1D: Proposed Drainage to Municipal System



#### Summary for Link E-1: Existing Drainage to Passaic River

Inflow Ar	ea =	327,405 sf, 93.75% Impervious,	Inflow Depth = 3.09"	for 2-Year event
Inflow	=	20.64 cfs @ 12.17 hrs, Volume=	84,226 cf	
Primary	=	20.64 cfs @ 12.17 hrs, Volume=	84,226 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



# Link E-1: Existing Drainage to Passaic River

#### Summary for Link P-1: Proposed Drainage to Passaic River

Inflow A	Area	=	327,405 sf,	, 63.05% Impervious,	Inflow Depth = 2.59	" for 2-Year event
Inflow	:	=	17.77 cfs @	12.17 hrs, Volume=	70,677 cf	
Primar	у	=	17.77 cfs @	12.17 hrs, Volume=	70,677 cf, At	ten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



## Link P-1: Proposed Drainage to Passaic River

<b>2019-10-28_Stormwater Calculations</b> Prepared by Microsoft HydroCAD® 10 00-22 s/n 06682 © 2018 HydroCAD Software Sol	NOAA 24-hr D 10-Year Rainfall=5.15" Printed 10/30/2019
Time span=0.00-30.00 hrs, dt=0.0	1 hrs, 3001 points
Runoff by SCS TR-20 method, UH=SCS	6, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind method - Pond	routing by Dyn-Stor-Ind method
Subcatchment E-1A: Existing Drainage to Runoff Area=52,	576 sf 82.07% Impervious Runoff Depth=4.57"
Tc=	10.0 min CN=80/98 Runoff=4.89 cfs 20,041 cf
SubcatchmentE-1B: Existing Drainage Runoff Area=141,6	68 sf 100.00% Impervious Runoff Depth=4.91"
Tc=	10.0 min CN=0/98 Runoff=13.91 cfs 58,001 cf
Subcatchment E-1C: Existing Drainageto Runoff Area=80,	227 sf 97.90% Impervious Runoff Depth=4.88"
Tc=	10.0 min CN=83/98 Runoff=7.84 cfs 32,621 cf
Subcatchment E-1D: Existing Drainage to Runoff Area=52,	934 sf 82.35% Impervious Runoff Depth=4.58"
Tc=	10.0 min CN=80/98 Runoff=4.93 cfs 20,201 cf
Subcatchment P-1A: Proposed Drainage to Runoff Area=50,	878 sf 41.10% Impervious Runoff Depth=3.80"
Tc=	10.0 min CN=80/98 Runoff=4.15 cfs 16,113 cf
Subcatchment P-1B: Proposed Drainage Runoff Area=142,	558 sf 72.74% Impervious Runoff Depth=4.40"
Tc=1	0.0 min CN=80/98 Runoff=12.89 cfs 52,247 cf
Subcatchment P-1C: Proposed Drainage to Runoff Area=81,	290 sf 71.62% Impervious Runoff Depth=4.38"
Tc=	10.0 min CN=80/98 Runoff=7.33 cfs 29,650 cf
Subcatchment P-1D: Proposed Drainage to Runoff Area=52,	679 sf 44.80% Impervious Runoff Depth=3.87"
Tc=	10.0 min CN=80/98 Runoff=4.35 cfs 16,990 cf
Link E-1: Existing Drainage to Passaic River	Inflow=31.57 cfs 130,864 cf Primary=31.57 cfs 130,864 cf
Link P-1: Proposed Drainage to Passaic River	Inflow=28.71 cfs 115,000 cf Primary=28.71 cfs 115,000 cf

### Total Runoff Area = 654,810 sf Runoff Volume = 245,864 cf Average Runoff Depth = 4.51" 21.60% Pervious = 141,443 sf 78.40% Impervious = 513,367 sf

### Summary for Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)

Runoff = 4.89 cfs @ 12.17 hrs, Volume= 20,041 cf, Depth= 4.57"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

	Area (sf)	CN	Description						
*	43,147	98	Impervious	Areas					
	9,429	80	>75% Gras	s cover, Go	ood, HSG D				
	52,576	95	Weighted A	verage					
	9,429	80	17.93% Per	17.93% Pervious Area					
	43,147	98	82.07% Imp	ervious Ar	ea				
-	Tc Length	Slop	e Velocity	Capacity	Description				
(mi	in) (feet)	(ft/f	ft) (ft/sec)	(cfs)					
10	).0				Direct Entry, Direct				

### Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)



### Summary for Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)

Runoff = 13.91 cfs @ 12.17 hrs, Volume= 58,001 cf, Depth= 4.91"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

	Are	ea (sf)	CN E	Description						
*	14	1,668	98 I	Impervious Areas						
	14	1,668	98 1	00.00% Im	Area					
	Tc	Length	Slope	Velocity	Capacity	Description				
(	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	10.0					Direct Entry, Direct				

Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)

Runoff = 7.84 cfs @ 12.17 hrs, Volume= 32,621 cf, Depth= 4.88"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

	Area (sf)	CN	Description						
*	78,544	98	Impervious	Areas					
	353	96	Gravel surfa	ace, HSG D					
	1,330	80	>75% Grass	s cover, Go	ood, HSG D				
	80,227	98	Weighted A	verage					
	1,683	83	2.10% Perv	ious Area					
	78,544	98	97.90% Imp	97.90% Impervious Area					
	Tc Length	Slop	be Velocity	Capacity	Description				
(m	in) (feet)	(ft/	ft) (ft/sec)	(cfs)					
10	).0				Direct Entry, Direct				

Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)



### Summary for Subcatchment E-1D: Existing Drainage to Municipal System

Runoff = 4.93 cfs @ 12.17 hrs, Volume= 20,201 cf, Depth= 4.58"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

	Area (sf)	CN	Description						
*	43,589	98	Impervious	Areas					
	9,345	80	>75% Gras	s cover, Go	bod, HSG D				
	52,934	95	Weighted A	Neighted Average					
	9,345	80	17.65% Pervious Area						
	43,589	98	82.35% Imp	pervious Ar	ea				
	Tc Length	Slope	e Velocity	Capacity	Description				
(m	in) (feet)	(ft/ft	) (ft/sec)	(cfs)					
10	0.0				Direct Entry, Direct				

### Subcatchment E-1D: Existing Drainage to Municipal System



### Summary for Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)

Runoff = 4.15 cfs @ 12.17 hrs, Volume= 16,113 cf, Depth= 3.80"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

	Area	(sf) CN	N De	Description						
*	20,9	09 98	8 Im	mpervious Areas						
	29,9	69 80	0 >7	>75% Grass cover, Good, HSG D						
	50,8	78 8	7 W	Weighted Average						
	29,9	69 80	0 58	58.90% Pervious Area						
	20,9	09 98	8 41	.10% Imp	ervious Are	ea				
	Tc Ler	ngth S	Slope	Velocity	Capacity	Description				
(I	<u>min) (f</u>	eet) (	<u>(ft/ft)</u>	(ft/sec)	(cfs)					
	10.0					Direct Entry, Direct				

#### Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)



### Summary for Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)

Runoff = 12.89 cfs @ 12.17 hrs, Volume= 52,247 cf, Depth= 4.40"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

Area (sf)	CN	Description	Description						
103,691	98	Impervious	mpervious Areas						
38,867	80	>75% Gras	>75% Grass cover, Good, HSG D						
142,558 93 Weighted Average									
38,867 80 27.26% Pervious Area				1					
103,691		72.74% Imp	pervious Are	ea					
			<b>o</b>						
c Length	Slop	e Velocity	Capacity	Description					
<u>1) (feet)</u>	(ft/ft	) (ft/sec)	(cfs)						
.0				Direct Entry, Direct					
I	Area (sf) 103,691 38,867 142,558 38,867 103,691 Tc Length n) (feet) .0	Area (sf) CN   103,691 98   38,867 80   142,558 93   38,867 80   103,691 98   Tc Length Slope   n) (feet) (ft/ft	Area (sf) CN Description   103,691 98 Impervious   38,867 80 >75% Grass   142,558 93 Weighted A   38,867 80 27.26% Per   103,691 98 72.74% Imp   Tc Length Slope Velocity   n) (feet) (ft/ft) (ft/sec)	Area (sf) CN Description   103,691 98 Impervious Areas   38,867 80 >75% Grass cover, Ge   142,558 93 Weighted Average   38,867 80 27.26% Pervious Area   103,691 98 72.74% Impervious Area   103 98 72.74% Impervious Area   103 98 72.74% Impervious Area   103 98 72.74% Impervious Area					

#### Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)



### Summary for Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)

Runoff = 7.33 cfs @ 12.17 hrs, Volume= 29,650 cf, Depth= 4.38"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

	Area (sf)	CN	Description							
*	58,221	98	Impervious	mpervious Areas						
	23,069	80	>75% Gras	>75% Grass cover, Good, HSG D						
	81,290	93	Weighted A	Weighted Average						
	23,069	80	28.38% Per	28.38% Pervious Area						
	58,221	98	71.62% Imp	ervious Ar	rea					
	Tc Length	Slop	e Velocity	Capacity	Description					
<u>(m</u>	in) (feet)	(ft/1	ft) (ft/sec)	(cfs)						
10	0.0				Direct Entry, Direct					

#### Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)



#### Summary for Subcatchment P-1D: Proposed Drainage to Municipal System

Runoff = 4.35 cfs @ 12.17 hrs, Volume= 16,990 cf, Depth= 3.87"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

	Area (sf)	CN	Description	Description					
*	23,598	98	Impervious	npervious Areas					
	29,081	80	>75% Gras	s cover, Go	bod, HSG D				
	52,679	88	Weighted A	Veighted Average					
	29,081	80	55.20% Per	55.20% Pervious Area					
	23,598	98	44.80% Imp	pervious Ar	ea				
Т	c Length	Slop	e Velocity	Capacity	Description				
(mir	n) (feet)	(ft/ft	) (ft/sec)	(cfs)					
10.	0				Direct Entry, Direct				

# Subcatchment P-1D: Proposed Drainage to Municipal System



#### Summary for Link E-1: Existing Drainage to Passaic River

Inflow /	Area =	327,405 st	, 93.75% Impervious,	Inflow Depth = 4.80"	for 10-Year event
Inflow	=	31.57 cfs @	12.17 hrs, Volume=	130,864 cf	
Primar	y =	31.57 cfs @	12.17 hrs, Volume=	130,864 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



# Link E-1: Existing Drainage to Passaic River

#### Summary for Link P-1: Proposed Drainage to Passaic River

Inflow A	Area =	327,405 sf,	, 63.05% In	npervious,	Inflow Depth =	4.21"	for 10-Year	event
Inflow	=	28.71 cfs @	12.17 hrs,	Volume=	115,000 c	f		
Primary	/ =	28.71 cfs @	12.17 hrs,	Volume=	115,000 c	f, Atter	i= 0%, Lag=	0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



## Link P-1: Proposed Drainage to Passaic River

2019-10-28_Stormwater Calculations Prepared by Microsoft HydroCAD® 10.00-22 s/n 06682 © 2018 HydroCAD Software	NOAA 24-hr D 25-Year Rainfall=6.34" Printed 10/30/2019 Solutions LLC Page 23
Time span=0.00-30.00 hrs, dt Runoff by SCS TR-20 method, UH= Reach routing by Dyn-Stor-Ind method - Pe	=0.01 hrs, 3001 points SCS, Split Pervious/Imperv. ond routing by Dyn-Stor-Ind method
Subcatchment E-1A: Existing Drainage to Runoff Area	=52,576 sf 82.07% Impervious Runoff Depth=5.74" Tc=10.0 min CN=80/98 Runoff=6.11 cfs 25,152 cf
Subcatchment E-1B: Existing Drainage Runoff Area=1	41,668 sf 100.00% Impervious Runoff Depth=6.10" Tc=10.0 min CN=0/98 Runoff=17.15 cfs 72,032 cf
Subcatchment E-1C: Existing Drainageto Runoff Area	=80,227 sf 97.90% Impervious Runoff Depth=6.07" Tc=10.0 min CN=83/98 Runoff=9.68 cfs 40,555 cf
Subcatchment E-1D: Existing Drainage to Runoff Area	=52,934 sf 82.35% Impervious Runoff Depth=5.75" Tc=10.0 min CN=80/98 Runoff=6.15 cfs 25,348 cf
Subcatchment P-1A: Proposed Drainage to Runoff Area	=50,878 sf 41.10% Impervious Runoff Depth=4.92" Tc=10.0 min CN=80/98 Runoff=5.34 cfs 20,844 cf
Subcatchment P-1B: Proposed Drainage Runoff Area=	142,558 sf 72.74% Impervious Runoff Depth=5.55" fc=10.0 min CN=80/98 Runoff=16.19 cfs 65,968 cf
Subcatchment P-1C: Proposed Drainage to Runoff Area	=81,290 sf 71.62% Impervious Runoff Depth=5.53" Tc=10.0 min CN=80/98 Runoff=9.21 cfs 37,465 cf
Subcatchment P-1D: Proposed Drainage to Runoff Area	=52,679 sf 44.80% Impervious Runoff Depth=4.99" Tc=10.0 min CN=80/98 Runoff=5.58 cfs 21,909 cf
Link E-1: Existing Drainage to Passaic River	Inflow=39.08 cfs 163,087 cf Primary=39.08 cfs 163,087 cf
Link P-1: Proposed Drainage to Passaic River	Inflow=36.33 cfs 146,186 cf Primary=36.33 cfs 146,186 cf

### Total Runoff Area = 654,810 sf Runoff Volume = 309,272 cf Average Runoff Depth = 5.67" 21.60% Pervious = 141,443 sf 78.40% Impervious = 513,367 sf

### Summary for Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)

Runoff = 6.11 cfs @ 12.17 hrs, Volume= 25,152 cf, Depth= 5.74"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

	Area (sf)	CN	Description							
*	43,147	98	Impervious	mpervious Areas						
	9,429	80	>75% Gras	>75% Grass cover, Good, HSG D						
	52,576	95 Weighted Average								
	9,429	80	17.93% Per	vious Area	3					
	43,147 98 82.07% Impervious Are			pervious Ar	ea					
				_						
-	Tc Length	Slop	e Velocity	Capacity	Description					
(mi	n) (feet)	(ft/f	t) (ft/sec)	(cfs)						
10	0.0				Direct Entry, Direct					

### Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)



#### Summary for Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)

Runoff = 17.15 cfs @ 12.17 hrs, Volume= 72,032 cf, Depth= 6.10"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

	Α	rea (sf)	CN I	Description					
*	1	41,668	98 I	Impervious Areas					
141,668		98 100.00% Impervious A			Area				
	Тс	Length	Slope	Velocity	Capacity	Description			
(n	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
1	0.0					Direct Entry, Direct			

Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)

Runoff = 9.68 cfs @ 12.17 hrs, Volume= 40,555 cf, Depth= 6.07"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

	Area (sf)	CN	Description	Description				
*	78,544	98	Impervious Areas	pervious Areas				
	353	96	Gravel surface, HSG	D				
	1,330	80	>75% Grass cover, G	ood, HSG D				
	80,227	98	Weighted Average	eighted Average				
	1,683	83	2.10% Pervious Area					
	78,544	98	97.90% Impervious A	rea				
	Tc Length	Slop	e Velocity Capacity	Description				
(	min) (feet)	(ft/	t) (ft/sec) (cfs)					
	10.0			Direct Entry, Direct				

Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)



### Summary for Subcatchment E-1D: Existing Drainage to Municipal System

Runoff = 6.15 cfs @ 12.17 hrs, Volume= 25,348 cf, Depth= 5.75"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

	Area (sf)	CN	Description						
*	43,589	98	Impervious	mpervious Areas					
	9,345	80	>75% Gras	s cover, Go	bod, HSG D				
	52,934	95	Weighted A	Veighted Average					
	9,345	80	17.65% Pervious Area						
	43,589	98	82.35% Imp	pervious Are	ea				
	Tc Length	Slope	Velocity	Capacity	Description				
(m	in) (feet)	(ft/ft)	(ft/sec)	(cfs)					
1(	0.0				Direct Entry, Direct				

### Subcatchment E-1D: Existing Drainage to Municipal System



### Summary for Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)

Runoff = 5.34 cfs @ 12.17 hrs, Volume= 20,844 cf, Depth= 4.92"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

	Area (sf)	CN	Description	Description						
*	20,909	98	Impervious	npervious Areas						
	29,969	80	>75% Gras	>75% Grass cover, Good, HSG D						
	50,878	3 87 Weighted Average								
	29,969	80	58.90% Per	vious Area	3					
	20,909	98	41.10% Imp	pervious Are	ea					
_				_						
-	Tc Length	Slop	e Velocity	Capacity	Description					
(mi	n) (feet)	(ft/1	t) (ft/sec)	(cfs)						
10	0.0				Direct Entry, Direct					

#### Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)



### Summary for Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)

Runoff = 16.19 cfs @ 12.17 hrs, Volume= 65,968 cf, Depth= 5.55"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

Area (sf)	CN	Description						
103,691	98	Impervious	mpervious Areas					
38,867	80	>75% Grass cover, Good, HSG D						
142,558	93	Weighted A	verage					
38,867	367 80 27.26% Pervious Area							
103,691 98 72.74% Impervious Are			pervious Are	ea				
			<b>o</b>					
c Length	Slop	e Velocity	Capacity	Description				
<u>1) (feet)</u>	(ft/ft	) (ft/sec)	(cfs)					
.0				Direct Entry, Direct				
I	Area (sf) 103,691 38,867 142,558 38,867 103,691 Tc Length n) (feet) .0	Area (sf) CN   103,691 98   38,867 80   142,558 93   38,867 80   103,691 98   Tc Length Slope   n) (feet) (ft/ft	Area (sf) CN Description   103,691 98 Impervious   38,867 80 >75% Grass   142,558 93 Weighted A   38,867 80 27.26% Per   103,691 98 72.74% Imp   Tc Length Slope Velocity   n) (feet) (ft/ft) (ft/sec)	Area (sf) CN Description   103,691 98 Impervious Areas   38,867 80 >75% Grass cover, Ge   142,558 93 Weighted Average   38,867 80 27.26% Pervious Area   103,691 98 72.74% Impervious Area   103 98 72.74% Impervious Area   103 98 72.74% Impervious Area   103 98 72.74% Impervious Area				

#### Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)



### Summary for Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)

Runoff = 9.21 cfs @ 12.17 hrs, Volume= 37,465 cf, Depth= 5.53"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

Area	ı (sf)	CN	Description						
58	,221	98	Impervious	mpervious Areas					
23	,069	80	>75% Gras	s cover, Go	bod, HSG D				
81	,290	93	Weighted A	Veighted Average					
23	,069	80	28.38% Per	28.38% Pervious Area					
58	,221	98	71.62% Imp	pervious Are	ea				
		~		<b>•</b> •					
TC Le	ength	Slope	e Velocity	Capacity	Description				
nin)	(feet)	(ft/ft	) (ft/sec)	(cfs)					
0.0					Direct Entry, Direct				
	Area 58 23 81 23 58 58 Tc Le in) 0.0	Area (sf) 58,221 23,069 81,290 23,069 58,221 Tc Length in) (feet) 0.0	Area (sf) CN   58,221 98   23,069 80   81,290 93   23,069 80   58,221 98   Tc Length Slope   in) (feet) (ft/ft	Area (sf) CN Description   58,221 98 Impervious   23,069 80 >75% Grass   81,290 93 Weighted A   23,069 80 28.38% Per   58,221 98 71.62% Imp   Tc Length Slope Velocity   in) (feet) (ft/ft) (ft/sec)	Area (sf)CNDescription58,22198Impervious Areas23,06980>75% Grass cover, Ge81,29093Weighted Average23,0698028.38% Pervious Area58,2219871.62% Impervious ArTcLengthSlopeVelocityin)(feet)(ft/ft)(ft/sec)0.0				

#### Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)



#### Summary for Subcatchment P-1D: Proposed Drainage to Municipal System

Runoff = 5.58 cfs @ 12.17 hrs, Volume= 21,909 cf, Depth= 4.99"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

	Area (sf)	CN	Description						
*	23,598	98	Impervious	Areas					
	29,081	80	>75% Gras	75% Grass cover, Good, HSG D					
	52,679	88	Weighted A	/eighted Average					
	29,081	80	55.20% Per	55.20% Pervious Area					
	23,598	98	44.80% Imp	pervious Are	ea				
	Tc Length	Slope	e Velocity	Capacity	Description				
(m	in) (feet)	(ft/ft	t) (ft/sec)	(cfs)					
10	0.0				Direct Entry, Direct				

# Subcatchment P-1D: Proposed Drainage to Municipal System



#### Summary for Link E-1: Existing Drainage to Passaic River

Inflow A	Area =	327,405 sf	, 93.75% Impervious,	Inflow Depth = 5.98"	for 25-Year event
Inflow	=	39.08 cfs @	12.17 hrs, Volume=	163,087 cf	
Primary	y =	39.08 cfs @	12.17 hrs, Volume=	163,087 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



# Link E-1: Existing Drainage to Passaic River

#### Summary for Link P-1: Proposed Drainage to Passaic River

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Inflow /	Area	=	327,405 sf,	63.05% Impervious,	Inflow Depth = 5.36	6" for 25-Year event
Inflow		=	36.33 cfs @	12.17 hrs, Volume=	146,186 cf	
Primar	у	=	36.33 cfs @	12.17 hrs, Volume=	146,186 cf, A	tten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



## Link P-1: Proposed Drainage to Passaic River

2019-10-28_Stormwater Calculations	NOAA 24-hr D 100-Year Rainfall=8.48" Printed 10/20/2010
HydroCAD® 10.00-22 s/n 06682 © 2018 HydroCAD So	ftware Solutions LLC Page 34
Time span=0.00-30.00 h Runoff by SCS TR-20 method, Reach routing by Dyn-Stor-Ind method	rs, dt=0.01 hrs, 3001 points UH=SCS, Split Pervious/Imperv. - Pond routing by Dyn-Stor-Ind method
Subcatchment E-1A: Existing Drainage to Runoff	Area=52,576 sf 82.07% Impervious Runoff Depth=7.85" Tc=10.0 min CN=80/98 Runoff=8.29 cfs 34,402 cf
Subcatchment E-1B: Existing Drainage Runoff A	rea=141,668 sf 100.00% Impervious Runoff Depth=8.24" Tc=10.0 min CN=0/98 Runoff=22.97 cfs 97,278 cf
Subcatchment E-1C: Existing Drainageto Runofi	Area=80,227 sf 97.90% Impervious Runoff Depth=8.20" Tc=10.0 min CN=83/98 Runoff=12.98 cfs 54,836 cf
Subcatchment E-1D: Existing Drainage to Runof	Area=52,934 sf 82.35% Impervious Runoff Depth=7.86" Tc=10.0 min CN=80/98 Runoff=8.35 cfs 34,663 cf
Subcatchment P-1A: Proposed Drainage to Runofi	Area=50,878 sf 41.10% Impervious Runoff Depth=6.97" Tc=10.0 min CN=80/98 Runoff=7.50 cfs 29,533 cf
Subcatchment P-1B: Proposed Drainage Runoff	Area=142,558 sf 72.74% Impervious Runoff Depth=7.65" Tc=10.0 min CN=80/98 Runoff=22.14 cfs 90,881 cf
Subcatchment P-1C: Proposed Drainage to Runoff	Area=81,290 sf 71.62% Impervious Runoff Depth=7.63" Tc=10.0 min CN=80/98 Runoff=12.60 cfs 51,659 cf
Subcatchment P-1D: Proposed Drainage to Runoff	Area=52,679 sf 44.80% Impervious Runoff Depth=7.05" Tc=10.0 min CN=80/98 Runoff=7.81 cfs 30,929 cf
Link E-1: Existing Drainage to Passaic River	Inflow=52.59 cfs 221,178 cf Primary=52.59 cfs 221,178 cf
Link P-1: Proposed Drainage to Passaic River	Inflow=50.05 cfs 203,003 cf Primary=50.05 cfs 203,003 cf

### Total Runoff Area = 654,810 sf Runoff Volume = 424,181 cf Average Runoff Depth = 7.77" 21.60% Pervious = 141,443 sf 78.40% Impervious = 513,367 sf

## Summary for Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)

Runoff = 8.29 cfs @ 12.17 hrs, Volume= 34,402 cf, Depth= 7.85"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

	Area (sf)	CN	Description							
*	43,147	98	Impervious	mpervious Areas						
	9,429	80	>75% Grass cover, Good, HSG D							
	52,576	95	95 Weighted Average							
	9,429	80	17.93% Per	17.93% Pervious Area						
	43,147	98	82.07% Imp	pervious Ar	rea					
Т	c Length	Slop	e Velocity	Capacity	Description					
(mir	<u>ı) (feet)</u>	(ft/f	t) (ft/sec)	(cfs)						
10.	0				Direct Entry, Direct					

### Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)



#### Summary for Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)

Runoff = 22.97 cfs @ 12.17 hrs, Volume= 97,278 cf, Depth= 8.24"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

	Are	ea (sf)	CN E	Description					
*	14	1,668	98 I	Impervious Areas					
	141,668 98 10			00.00% Im	npervious A	Area			
	Tc	Length	Slope	Velocity	Capacity	Description			
(	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	10.0					Direct Entry, Direct			

Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)

Runoff = 12.98 cfs @ 12.17 hrs, Volume= 54,836 cf, Depth= 8.20"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

	Area (sf)	CN	Description	Description					
*	78,544	98	Impervious	npervious Areas					
	353	96	Gravel surfa	ace, HSG D	)				
	1,330	80	>75% Grass cover, Good, HSG D						
	80,227	98	Weighted A	eighted Average					
	1,683	83	2.10% Perv	ious Area					
	78,544	98	97.90% Imp	ervious Are	ea				
	Tc Length	Slop	be Velocity	Capacity	Description				
(n	nin) (feet)	(ft/	ft) (ft/sec)	(cfs)					
1	0.0				Direct Entry, Direct				

Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)



### Summary for Subcatchment E-1D: Existing Drainage to Municipal System

Runoff = 8.35 cfs @ 12.17 hrs, Volume= 34,663 cf, Depth= 7.86"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

	Area (sf)	CN	Description						
*	43,589	98	Impervious	Areas					
	9,345	80	>75% Gras	75% Grass cover, Good, HSG D					
	52,934	95	Weighted A	Veighted Average					
	9,345	80	17.65% Per	17.65% Pervious Area					
	43,589	98	82.35% Imp	pervious Ar	ea				
Т	c Length	Slop	e Velocity	Capacity	Description				
(mir	) (feet)	(ft/ft	) (ft/sec)	(cfs)					
10.	0				Direct Entry, Direct				

### Subcatchment E-1D: Existing Drainage to Municipal System



## Summary for Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)

Runoff = 7.50 cfs @ 12.17 hrs, Volume= 29,533 cf, Depth= 6.97"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

	Area (sf)	CN	Description						
*	20,909	98	Impervious	mpervious Areas					
	29,969	80	>75% Grass cover, Good, HSG D						
	50,878	87	Weighted Average						
	29,969	80	58.90% Per	58.90% Pervious Area					
	20,909	98	41.10% Imp	ervious Ar	ea				
	Tc Length	Slop	e Velocity	Capacity	Description				
(m	in) (feet)	(ft/1	t) (ft/sec)	(cfs)					
10	0.0				Direct Entry, Direct				

#### Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)



## Summary for Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)

Runoff = 22.14 cfs @ 12.17 hrs, Volume= 90,881 cf, Depth= 7.65"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

	Area (sf)	CN	Description						
*	103,691	98	Impervious	Impervious Areas					
	38,867	80	>75% Gras	>75% Grass cover, Good, HSG D					
	142,558	142,558 93 Weighted Average							
	38,867	80	27.26% Per	vious Area	3				
	103,691	98	72.74% Imp	pervious Ar	rea				
				_					
	Tc Length	Slop	be Velocity	Capacity	Description				
	(min) (feet)	) (ft/	ft) (ft/sec)	(cfs)					
	10.0				Direct Entry, Direct				

#### Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)


### Summary for Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)

Runoff = 12.60 cfs @ 12.17 hrs, Volume= 51,659 cf, Depth= 7.63"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

	Area (sf)	CN	Description		
*	58,221	98	Impervious	Areas	
	23,069	80	>75% Gras	s cover, Go	ood, HSG D
	81,290	93	Weighted A	verage	
	23,069	80	28.38% Per	vious Area	a
	58,221	98	71.62% Imp	pervious Ar	ea
_					
	Fc Length	Slop	e Velocity	Capacity	Description
(mi	n) (feet)	(ft/1	t) (ft/sec)	(cfs)	
10	.0				Direct Entry, Direct

#### Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)



#### Summary for Subcatchment P-1D: Proposed Drainage to Municipal System

Runoff = 7.81 cfs @ 12.17 hrs, Volume= 30,929 cf, Depth= 7.05"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

	Area (sf)	CN	Description		
*	23,598	98	Impervious	Areas	
	29,081	80	>75% Grass	s cover, Go	bod, HSG D
	52,679	88	Weighted A	verage	
	29,081	80	55.20% Per	vious Area	l
	23,598	98	44.80% Imp	ervious Are	ea
	Tc Length	Slop	e Velocity	Capacity	Description
(m	nin) (feet)	(ft/f	t) (ft/sec)	(cfs)	
1	0.0				Direct Entry, Direct

### Subcatchment P-1D: Proposed Drainage to Municipal System



#### Summary for Link E-1: Existing Drainage to Passaic River

Inflow /	Area =	327,405 sf, 93.75% Impervious	, Inflow Depth = 8.11"	for 100-Year event
Inflow	=	52.59 cfs @ 12.17 hrs, Volume=	221,178 cf	
Primary	y =	52.59 cfs @ 12.17 hrs, Volume=	221,178 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



#### Link E-1: Existing Drainage to Passaic River

#### Summary for Link P-1: Proposed Drainage to Passaic River

Inflow /	Area	=	327,405 sf,	63.05% Impervious,	Inflow Depth = 7.44"	for 100-Year event
Inflow	=	=	50.05 cfs @	12.17 hrs, Volume=	203,003 cf	
Primar	у =	=	50.05 cfs @	12.17 hrs, Volume=	203,003 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



#### Link P-1: Proposed Drainage to Passaic River

### Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)		Dns Line No.	Junction Type
1	1A - E100	6.39	18	Cir	25.000	250.91	251.04	0.520	251.97	252.10	0.37	252.46	6	End	Grate
2	E100 - D100	4.39	18	Cir	12.000	251.04	251.83	6.583	252.46	252.63	n/a	252.63	3 j	1	Manhole
3	D100 - R100	4.39	15	Cir	10.000	253.95	254.05	1.000	254.71	254.90	0.36	254.90	0	2	None
4	R100 - R101	4.14	15	Cir	68.000	254.05	254.73	1.000	254.90	255.55	n/a	255.55	5 j	3	None
5	R101 - D101	3.86	15	Cir	24.000	254.73	254.97	1.000	255.55	255.76	n/a	255.76	6 j	4	Manhole
6	D101 - YD01	3.87	15	Cir	30.000	254.97	255.27	1.000	255.76	256.06	0.17	256.06	6	5	Grate
7	YD01 - D102	3.70	15	Cir	64.000	255.27	255.91	1.000	256.06	256.69	n/a	256.69	9 j	6	Manhole
8	D102 - D103	1.39	15	Cir	24.000	255.91	256.15	1.000	256.69	256.61	0.09	256.61	1	7	Grate
9	D103 - D104	0.53	15	Cir	70.000	256.15	256.85	1.000	256.61	257.13	n/a	257.13	3 j	8	Grate
10	R100 - BLDG	0.30	6	Cir	5.000	254.05	254.15	2.000	254.90*	254.91*	0.04	254.95	5	3	None
11	R101 - BLDG	0.30	6	Cir	5.000	254.73	254.83	2.000	255.55*	255.57*	0.04	255.60	0	4	None
12	D102 - D105	2.38	15	Cir	155.000	255.91	259.79	2.503	256.69	260.41	n/a	260.41	1 j	7	Grate
13	1B - D200	16.30	42	Cir	125.000	248.21	248.52	0.248	249.58	249.89	0.34	250.23	3	End	Manhole
14	D200 - D201	15.79	24	Cir	11.000	250.25	250.36	1.000	251.48	251.79	n/a	251.79	9	13	Manhole
15	D201 -D202	14.77	18	Cir	20.000	250.36	250.56	1.000	251.86*	252.26*	1.09	253.34	4	14	Manhole
16	D202 - D203	10.37	18	Cir	56.000	250.56	251.12	1.000	253.34*	253.89*	0.27	254.16	6	15	Combination
17	D203 - D204	9.38	18	Cir	46.000	251.12	251.58	1.000	254.16*	254.52*	0.44	254.96	6	16	Manhole
18	D204 - D205	8.88	18	Cir	83.000	251.58	252.41	1.000	254.96*	255.56*	0.20	255.75	5	17	Grate
19	D205 - D206	8.56	18	Cir	70.000	252.41	253.11	1.000	255.75*	256.22*	0.18	256.40	0	18	Grate
20	D206 - D207	7.29	18	Cir	114.000	253.11	254.25	1.000	256.40*	256.95*	0.26	257.22	2	19	Manhole
21	D207 - D208	4.66	15	Cir	35.000	254.25	254.60	1.000	257.22*	257.40*	0.25	257.65	5	20	Grate
22	D208 - D209	3.15	15	Cir	39.000	254.60	254.99	1.000	257.65*	257.75*	0.08	257.82	2	21	Manhole
23	D209 - D210	3.21	15	Cir	86.000	254.99	255.85	1.000	257.82*	258.04*	0.11	258.14	4	22	Manhole
24	D210 - D211	1.42	15	Cir	53.000	255.85	256.38	1.000	258.14*	258.17*	0.02	258.19	9	23	Combination
Project I	File: 2019-10-30_Pipe Sizing.stm	Number of lines: 138 Run Date: 10/31/2019					/2019								
NOTES															

NOTES: Return period = 25 Yrs. ; \*Surcharged (HGL above crown). ; j - Line contains hyd. jump.

Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)	t	Dns Line No.	Junction Type
25	D200 - R200	0.59	6	Cir	5.000	257.36	257.41	1.000	257.80	257.85	0.16	258.0	1	13	None
26	R200 - R201	0.59	6	Cir	11.000	257.41	257.52	1.000	258.01*	258.13*	0.14	258.2	8	25	None
27	R201 - R202	0.30	6	Cir	68.000	257.52	258.20	1.000	258.28	258.52	0.08	258.6	0	26	None
28	R202 - BLDG	0.30	6	Cir	5.000	258.20	258.25	1.000	258.60	258.53	0.11	258.5	3	27	None
29	R201 - BLDG	0.30	6	Cir	5.000	257.52	257.57	1.000	258.28*	258.29*	0.04	258.3	3	26	None
30	D201 - D212	1.18	15	Cir	48.000	256.77	257.25	1.000	257.13	257.68	0.16	257.6	8	14	Combination
31	D202 - YD09	1.12	6	Cir	22.170	257.30	257.52	0.992	257.80*	258.69*	0.75	259.4	5	15	Grate
32	YD09 - YD10	0.57	6	Cir	68.380	257.52	258.20	0.994	259.45*	260.15*	0.20	260.3	5	31	Grate
33	YD10 - BLDG	0.30	6	Cir	5.000	258.20	258.25	1.000	260.35*	260.36*	0.04	260.4	0	32	None
34	YD09 - BLDG	0.30	6	Cir	5.000	257.57	257.62	1.000	259.45*	259.46*	0.04	259.5	0	31	None
35	D202 - YD03	3.83	15	Cir	84.000	257.17	258.01	1.000	257.86	258.80	n/a	258.8	0	15	Grate
36	YD03 - YD04	3.02	15	Cir	68.000	258.01	258.69	1.000	258.80	259.39	n/a	259.3	9 j	35	Grate
37	YD04 - D213	1.91	15	Cir	30.000	258.69	258.99	1.000	259.39	259.54	n/a	259.5	4 j	36	Manhole
38	D213 - YD05	0.23	6	Cir	16.000	258.99	259.15	1.000	259.54	259.56	0.01	259.5	7	37	Grate
39	YD05 - YD06	0.12	6	Cir	26.000	259.15	259.41	1.000	259.57	259.58	n/a	259.5	8 j	38	Grate
40	YD03 - BLDG	0.30	6	Cir	35.000	258.01	258.36	1.000	258.80	258.86	0.04	258.9	0	35	None
41	YD03 - BLDG	0.30	6	Cir	11.000	258.01	258.12	1.000	258.80*	258.83*	0.02	258.8	5	35	None
42	YD03 - BLDG	0.30	6	Cir	5.000	258.12	258.17	1.000	258.85*	258.86*	0.04	258.9	0	41	None
43	YD04 - BLDG	0.36	6	Cir	35.000	258.69	259.04	1.000	259.39	259.52	0.05	259.5	8	36	None
44	YD04 - BLDG	0.36	6	Cir	11.000	258.69	258.80	1.000	259.39*	259.43*	0.02	259.4	6	36	None
45	YD04 - BLDG	0.36	6	Cir	5.000	258.80	258.85	1.000	259.46*	259.48*	0.05	259.5	3	44	None
46	D213 - R203	1.69	15	Cir	24.000	260.08	260.32	1.000	260.52	260.84	n/a	260.8	4	37	None
47	R203 - YD07	1.46	15	Cir	24.000	260.32	260.56	1.000	260.84	261.04	n/a	261.04	4 j	46	Grate
48	YD07 - YD08	1.36	15	Cir	21.000	260.56	260.77	1.000	261.04	261.23	n/a	261.2	3 j	47	Grate
Project I	Project File: 2019-10-30_Pipe Sizing.stm Run Date: 10/31/2019														
NOTES:	Return period = 25 Yrs. ; *Surcha	arged (HGL	above crown)	. ; j - Line	e contains h	yd. jump.			1			I			

Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Jun (ft)	- ct	Dns Line No.	Junction Type
49	YD08 - D214	1.20	15	Cir	16.000	260.77	260.93	1.000	261.23	261.36	n/a	261.	.36 j	48	Combination
50	R203 - BLDG	0.24	6	Cir	6.000	260.32	260.38	1.000	260.84	260.84	0.03	260.	.87	46	None
51	D204 - D215	0.90	15	Cir	21.000	251.58	251.79	1.000	254.96*	254.97*	0.00	254.	.97	17	Grate
52	D215 - D216	0.41	15	Cir	70.000	251.79	252.49	1.000	254.97*	254.97*	0.00	254.	.98	51	Grate
53	D207 - D217	2.68	15	Cir	43.000	261.50	261.93	1.000	262.06	262.59	n/a	262.	.59	20	Combination
54	D217 - D218	1.57	15	Cir	60.000	261.93	262.53	1.000	262.59	263.03	n/a	263.	.03 j	53	Manhole
55	D218 - R204	1.37	12	Cir	13.000	264.60	264.73	1.000	265.03	265.22	n/a	265.	.22	54	None
56	R204 - R205	1.14	12	Cir	42.000	264.73	265.15	1.000	265.22	265.60	n/a	265.	.60 j	55	None
57	R205 - R206	0.91	12	Cir	42.000	265.15	265.57	1.000	265.60	265.97	n/a	265.	.97 j	56	None
58	R206 - BLDG	0.24	6	Cir	10.000	265.57	265.67	1.000	265.97	265.92	n/a	265.	.92	57	None
59	R204 - BLDG	0.24	6	Cir	10.000	264.73	264.83	1.000	265.22	265.08	n/a	265.	.08	55	None
60	R205 - BLDG	0.24	6	Cir	10.000	265.15	265.25	1.000	265.60	265.50	n/a	265.	.50	56	None
61	D217 - R208	0.22	6	Cir	69.000	264.60	265.29	1.000	264.82	265.53	n/a	265.	.53	54	None
62	R208 - YD11	0.23	6	Cir	16.000	265.29	265.45	1.000	265.53	265.69	0.05	265.	.69	61	Grate
63	YD11 - YD12	0.17	6	Cir	32.000	265.45	265.77	1.000	265.69	265.98	n/a	265.	.98 j	62	Grate
64	YD12 - YD13	0.11	6	Cir	32.000	265.77	266.09	1.000	265.98	266.26	n/a	266.	.26 j	63	Grate
65	D210 - R209	1.84	12	Cir	29.000	258.75	259.04	1.000	259.26	259.62	n/a	259.	.62	23	None
66	R209 - R210	0.96	12	Cir	68.000	259.04	259.72	1.000	259.62	260.13	n/a	260.	.13 j	65	None
67	R209 - BLDG	0.30	6	Cir	37.000	259.04	259.41	1.000	259.62	259.73	0.08	259.	.81	65	None
68	R209 - YD14	0.61	6	Cir	11.000	259.04	259.15	1.000	259.62*	259.75*	0.08	259.	.82	65	Grate
69	YD14 - BLDG	0.30	6	Cir	10.000	259.15	259.25	1.000	259.82*	259.85*	0.04	259.	.89	68	None
70	R210 - BLDG	0.30	6	Cir	37.000	259.72	260.09	1.000	260.13	260.37	n/a	260.	.37 j	66	None
71	R210 - YD15	0.66	6	Cir	11.000	259.72	259.83	1.000	260.22	260.33	0.09	260.	.42	66	Grate
72	YD15 - BLDG	0.30	6	Cir	10.000	259.83	259.93	1.000	260.42	260.43	0.04	260.	.47	71	None
Project I	Project File: 2019-10-30_Pipe Sizing.stm Run Date: 10/31/2019													/2019	
NOTES	Return period = 25 Yrs. ; *Surcha	arged (HG	L above crown)	.;j-Line	contains h	ıyd. jump.			1						

Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junc (ft)	t	Dns Line No.	Junction Type
73	R206 - R207	0.67	12	Cir	30.000	265.57	265.87	1.000	265.97	266.21	n/a	266.2	21 j	57	None
74	R207 - D219	0.68	12	Cir	25.000	265.87	266.12	1.000	266.21	266.46	n/a	266.4	16	73	Grate
75	1C - D300	9.63	15	Cir	50.000	244.75	245.00	0.500	246.00*	247.11*	1.12	248.2	23	End	Grate
76	D300 - D301	9.31	15	Cir	24.000	248.50	249.22	3.000	249.37	250.39	n/a	250.3	39	75	Manhole
77	D301 - D302	8.15	15	Cir	69.000	249.22	250.60	2.000	250.39	251.72	n/a	251.7	72 j	76	Manhole
78	D302 - D303	7.36	15	Cir	30.000	250.75	251.05	1.000	252.00*	252.39*	0.80	253.1	19	77	Grate
79	D303 - D304	4.08	15	Cir	114.000	251.05	252.19	1.000	253.19*	253.65*	0.26	253.9	91	78	Grate
80	D304 - D305	3.24	15	Cir	66.000	252.19	252.85	1.000	253.91	254.06	0.06	254.1	12	79	Grate
81	D305 - D306	2.44	15	Cir	62.000	252.85	253.47	1.000	254.12	254.16	0.10	254.2	26	80	Grate
82	D306 - D307	1.89	15	Cir	70.000	253.47	254.17	1.000	254.26	254.72	n/a	254.7	72 j	81	Grate
83	D307 - D308	1.40	15	Cir	62.000	254.17	254.79	1.000	254.72	255.26	n/a	255.2	26 j	82	Grate
84	D308 - D309	1.12	15	Cir	70.000	254.79	255.49	1.000	255.26	255.91	n/a	255.9	91 j	83	Grate
85	D301 - R300	0.60	8	Cir	10.000	254.17	254.27	1.000	254.50	254.63	n/a	254.6	33	76	None
86	R300 - R301	0.30	8	Cir	21.000	254.27	254.48	1.000	254.63	254.73	n/a	254.7	73 j	85	None
87	R301 - BLDG	0.30	6	Cir	5.000	254.48	254.53	1.000	254.74	254.81	0.11	254.8	31	86	None
88	R300 - R302	0.30	8	Cir	48.000	254.27	254.75	1.000	254.63	255.00	n/a	255.0	)0 j	85	None
89	R302 - BLDG	0.30	6	Cir	5.000	254.75	254.80	1.000	255.01	255.08	0.11	255.0	)8	88	None
90	D301 - YD16	0.69	8	Cir	32.000	252.22	252.54	1.000	252.58	252.93	0.25	252.9	93	76	Grate
91	YD16 - YD17	0.35	8	Cir	68.000	252.54	253.22	1.000	252.93	253.50	n/a	253.5	50 j	90	Grate
92	YD17 - BLDG	0.30	6	Cir	7.000	253.22	253.29	1.000	253.50	253.57	0.11	253.5	57	91	None
93	YD16 - BLDG	0.30	6	Cir	7.000	252.54	252.61	1.000	252.93	252.89	0.11	252.8	39	90	None
94	D302 - D310	0.89	15	Cir	42.000	254.58	255.00	1.000	254.89	255.37	n/a	255.3	37	77	Combination
95	D303 - R303	2.78	15	Cir	75.000	253.85	254.60	1.000	254.42	255.27	n/a	255.2	27	78	None
96	R303 - R304	1.71	15	Cir	69.000	254.60	255.29	1.000	255.27	255.81	n/a	255.8	81 j	95	None
Project F	ile: 2019-10-30_Pipe Sizing.stm	1	<u> </u>	<u> </u>		1	1	1	Number o	f lines: 138	1		Run [	Date: 10/31	/2019
NOTES:	Return period = 25 Yrs. ; *Surcha	arged (HGL	. above crown)	.;j-Line	contains h	yd. jump.			1						

Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns Line No.	Junction Type
97	R304 - R305	0.87	15	Cir	26.000	255.29	255.55	1.000	255.81	255.91	n/a	255.91 j	96	None
98	R303 - BLDG	0.30	6	Cir	37.000	254.60	254.97	1.000	255.27	255.37	0.05	255.42	95	None
99	R303 - YD18	0.82	6	Cir	11.000	254.60	254.71	1.000	255.27*	255.50*	0.14	255.64	95	Grate
100	YD18 - BLDG	0.30	6	Cir	10.000	254.71	254.81	1.000	255.64*	255.67*	0.04	255.71	99	None
101	R304 - BLDG	0.30	6	Cir	37.000	255.29	255.66	1.000	255.81	255.94	n/a	256.05 j	96	None
102	R304 - YD19	0.57	6	Cir	11.000	255.29	255.40	1.000	255.81	255.90	0.07	255.96	96	Grate
103	YD19 - BLDG	0.30	6	Cir	10.000	255.40	255.50	1.000	255.96	255.99	0.04	256.03	102	None
104	R305 - YD20	0.27	6	Cir	37.000	255.55	255.92	1.000	255.91	256.18	n/a	256.18 j	97	Grate
105	YD20 - YD21	0.22	6	Cir	25.000	255.92	256.17	1.000	256.18	256.40	n/a	256.40 j	104	Grate
106	R305 - YD22	0.60	6	Cir	21.000	255.55	255.76	1.000	256.01	256.22	0.08	256.30	97	Grate
107	YD22 - YD23	0.27	6	Cir	25.000	255.76	256.01	1.000	256.30	256.34	0.03	256.37	106	Grate
108	YD23 - YD24	0.11	6	Cir	18.000	256.01	256.19	1.000	256.37	256.36	0.06	256.36	107	Grate
109	1D - D400	3.96	15	Cir	100.000	236.31	243.81	7.500	236.71	244.61	0.52	244.61	End	Grate
110	D400 - D401	2.56	15	Cir	10.000	244.67	244.97	3.000	245.08	245.61	0.25	245.61	109	Manhole
111	D401 - R400	1.88	15	Cir	135.000	248.00	252.05	3.000	248.35	252.60	n/a	252.60	110	None
112	R400 - R401	1.17	15	Cir	132.000	252.05	256.01	3.000	252.60	256.44	n/a	256.44 j	111	None
113	D401 - R402	0.75	10	Cir	18.000	248.00	251.60	20.000	248.16	251.98	0.15	251.98	110	None
114	R402 - R403	0.30	6	Cir	31.000	251.60	251.91	1.000	251.98	252.19	n/a	252.19 j	113	None
115	R403 - BLDG	0.30	6	Cir	3.000	251.91	251.94	1.000	252.19	252.22	0.11	252.22	114	None
116	R402 - R404	0.46	6	Cir	37.000	251.60	251.97	1.000	251.98	252.31	0.16	252.31	113	None
117	R404 - R405	0.16	6	Cir	17.000	251.97	252.14	1.000	252.31	252.34	n/a	252.34 j	116	None
118	R405 - YD25	0.16	6	Cir	11.000	252.14	252.25	1.000	252.34	252.45	0.08	252.45	117	Grate
119	YD25 - YD26	0.11	6	Cir	25.000	252.25	252.50	1.000	252.45	252.66	n/a	252.66 j	118	Grate
120	R404 - BLDG	0.30	6	Cir	7.000	251.97	252.04	1.000	252.31	252.32	0.11	252.32	116	None
Project F									Number o	f lines: 138		Run	Date: 10/31	/2019

NOTES: Return period = 25 Yrs. ; \*Surcharged (HGL above crown). ; j - Line contains hyd. jump.

Line No.	Line ID	Flow rate (cfs)	Line Size (in)	Line shape	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line Slope (%)	HGL Down (ft)	HGL Up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns Line No.	Junction Type
121	R400 - R406	0.78	10	Cir	15.000	252.05	252.20	1.000	252.60	252.59	n/a	252.59 j	111	None
122	R406 - R407	0.30	6	Cir	34.000	252.20	252.54	1.000	252.59	252.82	n/a	252.82 j	121	None
123	R407 - BLDG	0.30	6	Cir	10.000	252.54	252.64	1.000	252.82	252.92	0.11	252.92	122	None
124	R406 - R408	0.49	6	Cir	34.000	252.20	252.54	1.000	252.59	252.90	n/a	253.06 j	121	None
125	R408 - R409	0.19	6	Cir	14.000	252.54	252.68	1.000	253.06	253.07	0.02	253.09	124	None
126	R409 - YD27	0.19	6	Cir	15.000	252.68	252.83	1.000	253.09	253.05	0.09	253.05	125	Grate
127	YD27 - YD28	0.13	6	Cir	25.000	252.83	253.08	1.000	253.05	253.26	n/a	253.26 j	126	Grate
128	R408 - BLDG	0.30	6	Cir	10.000	252.54	252.64	1.000	253.06	253.08	0.04	253.12	124	None
129	R401 - R410	1.17	10	Cir	15.000	256.01	256.16	1.000	256.44	256.64	0.20	256.64	112	None
130	R410 - R411	0.30	6	Cir	34.000	256.16	256.50	1.000	256.64	256.78	n/a	256.78 j	129	None
131	R411 - BLDG	0.30	6	Cir	10.000	256.50	256.60	1.000	256.78	256.88	0.11	256.88	130	None
132	R410 - R412	0.89	6	Cir	34.000	256.16	256.50	1.000	256.66*	257.51*	0.32	257.83	129	None
133	R412 - R413	0.60	6	Cir	14.000	256.50	256.64	1.000	257.83*	257.99*	0.11	258.10	132	None
134	R413 - YD29	0.60	6	Cir	15.000	256.64	256.79	1.000	258.10*	258.27*	0.16	258.43	133	Grate
135	YD29 - YD30	0.50	6	Cir	25.000	256.79	257.04	1.000	258.43*	258.63*	0.05	258.68	134	Grate
136	YD30 - YD31	0.35	6	Cir	32.000	257.04	257.36	1.000	258.68*	258.81*	0.03	258.84	135	Grate
137	YD31 - YD32	0.21	6	Cir	37.000	257.36	257.73	1.000	258.84*	258.89*	0.02	258.90	136	Grate
138	R412 - BLDG	0.30	6	Cir	10.000	256.50	256.60	1.000	257.83*	257.86*	0.04	257.89	132	None
Project F	Project File: 2019-10-30_Pipe Sizing.stm Number of lines: 138 Run Date: 10/31/2019													
NOTES:	NOTES: Return period = 25 Yrs. ; *Surcharged (HGL above crown). ; j - Line contains hyd. jump.													

Line Profile (Line 1) - 1A - E100





Line Profile (Line 3) - D100 - R100

























### Line Profile (Line 15) - D201 - D202



Line Profile (Line 16) - D202 - D203





Line Profile (Line 18) - D204 - D205



Line Profile (Line 19) - D205 - D206



Line Profile (Line 20) - D206 - D207



### Line Profile (Line 21) - D207 - D208





Line Profile (Line 23) - D209 - D210








Line Profile (Line 27) - R201 - R202







Line Profile (Line 30) - D201 - D212



Line Profile (Line 31) - D202 - YD09



















Line Profile (Line 40) - YD03 - BLDG







Line Profile (Line 43) - YD04 - BLDG







Line Profile (Line 46) - D213 - R203













Line Profile (Line 52) - D215 - D216



Line Profile (Line 53) - D207 - D217



Line Profile (Line 54) - D217 - D218



Line Profile (Line 55) - D218 - R204









Line Profile (Line 59) - R204 - BLDG





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Line Profile (Line 61) - D217 - R208










Line Profile (Line 66) - R209 - R210





















Line Profile (Line 76) - D300 - D301



Line Profile (Line 77) - D301 - D302







Line Profile (Line 80) - D304 - D305



Line Profile (Line 81) - D305 - D306







Line Profile (Line 84) - D308 - D309



Line Profile (Line 85) - D301 - R300

















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Line Profile (Line 95) - D303 - R303



Line Profile (Line 96) - R303 - R304



Line Profile (Line 97) - R304 - R305








Line Profile (Line 101) - R304 - BLDG







Line Profile (Line 104) - R305 - YD20



Line Profile (Line 105) - YD20 - YD21









Line Profile (Line 109) - 1D - D400





Line Profile (Line 111) - D401 - R400



Line Profile (Line 112) - R400 - R401



Line Profile (Line 113) - D401 - R402



Line Profile (Line 114) - R402 - R403





Line Profile (Line 116) - R402 - R404



Line Profile (Line 117) - R404 - R405











Line Profile (Line 122) - R406 - R407











Line Profile (Line 126) - R409 - YD27



Line Profile (Line 127) - YD27 - YD28


























## APPENDIX D DRAINAGE AREA MAPS

## **INVENTORY**

SHEET I OF 3: EXISTING DRAINAGE AREA MAP SHEET 2 OF 3: PROPOSED DRAINAGE AREA MAP SHEET 3 OF 3: PROPOSED INLET AREA MAP







SYMBOL

## DESCRIPTION

EXISTING DRAINAGE AREA SOIL TYPE BOUNDARY

EXISTING GRASSED AREA



60' GRAPHIC SCALE IN FEET I" = 30'







## SYMBOL

DL DESCRIPTION

PROPOSED DRAINAGE AREA

soil type boundary

PROPOSED GRASSED AREA



0' 30' GRAPHIC SCALE IN FEET I" = 30'

60'











