## STONEFIELD

## Stormwater Management Report Enclave at Millington

Proposed Mixed-Use Residential \& Commercial Development BLock I230I, LOT I \& BLOck IOI00, LOt 7.01 50 division Avenue Millington, Township of Long Hill Morris County, New Jersey

Prepared For: Prism Millington, LLC

Prepared by: Stonefield Engineering \& Design, tLC

92 Park Avenue
Rutherford, New Jersey

Report Date:
OCtober 25, 2019


Chuck D. Olive, Pe, PP, PTOE
NJ PE LICENSE \#46719
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## REPORT CONTENTS

I. 0 Project Description ..... I
2.0 Existing Conditions ..... I
EXISTING SITE DEVELOPMENT .....
EXISTING TOPOGRAPHY ..... 2
PROJECT SITE SOILS ..... 2
EXISTING ENVIRONMENTAL INVENTORY ..... 2
3.0 Proposed Conditions ..... 3
PROPOSED SITE DEVELOPMENT ..... 3
PROPOSED TOPOGRAPHY ..... 3
ANTICIPATED ENVIRONMENTAL INVENTORY IMPACTS ..... 3
4.0 Stormwater Management Methodology \& Parameters. ..... 3
HYDROLOGIC METHODOLOGY ..... 3
HYDRAULIC METHODOLOGY ..... 4
5.0 STORMWATER ANALYSIS ..... 4
EXISTING DRAINAGE AREAS ..... 4
PROPOSED DRAINAGE AREAS ..... 5
STORMWATER MANAGEMENT DESIGN PARAMETERS ..... 6
STORMWATER RUNOFF QUANTITY ..... 7
GROUNDWATER RECHARGE ..... 8
STORMWATER PIPE CONVEYANCE SYSTEM ..... 8
6.0 StORMWATER FACILITY Operations \& Maintenance ..... 9
7.0 Erosion \& Sediment Control ..... 9
8.0 CONCLUSIONS ..... 10
9.0 References. ..... 10

## APPENDICES

Project Figures ..... A
USGS LOCATION MAP ..... Figure I
Aerial Map ..... Figure 2
Tax \& Zoning Map ..... Figure 3
Overall Site Plan (Not to Scale) ..... Figure 4
NRCS Soils Report ..... B
Hydrologic \& Hydraulic Calculations ..... C
NOAA Rainfall Data ..... C-I
Hydrocad Node Schematic Diagram ..... C-2
HydroCAD Hydrologic Calculations ..... C-3
2-Year Storm Event Hydrographs
10-Year Storm Event Hydrographs
25-Year Storm Event Hydrographs
100-Year Storm Event Hydrographs
Hydraflow Pipe Network Schematic Diagram ..... C-4
HydraFlow Hydraulic Calculations ..... C-5
Pipe Network Summary Chart
Individual Pipe Segment Profiles
Drainage Area Maps ..... D
Existing Drainage Area Map ..... I OF 2
Proposed Drainage Area Map ..... 2 OF 2

## I. 0 Project Description

Prism Millington, LLC is proposing to redevelop Block 12301, Lot I and Block 10100, Lot 7.01 (herein referred to as the "project site") to accommodate fourteen multi-family residential buildings ( 140 units total), one commercial building (approximately $4,992 \mathrm{SF}$ ), and multiple amenity areas for residents (consisting of a multiple patios throughout the project site, community building and a pool). Additional improvements include multiple patio areas located throughout the project site, off-street parking lots, lighting, and landscaping. The subject property is located within the Township of Long Hill and is bounded by an NJ Transit Railroad to the north, Division Avenue to the east, Stone House Road to the south, and the Passaic River to the west.

Refer to APPENDIX A for project maps of the subject site.

The total project area is $518,332 \mathrm{SF}$ ( 11.90 acres), the impervious surfaces has been reduced by 101,084 SF ( $\mathbf{2 . 3 2}$ acres), and the total area of disturbance is 339,160 SF (7.79 acres).

This Stormwater Management Report has been prepared to analyze the potential stormwater runoff impacts of the proposed project and discuss the measures proposed to conform to the stormwater management requirements set forth by the Township of Long Hill, Morris County Soil Conservation District, and the New Jersey Department of Environmental Protection (NJDEP).

### 2.0 Existing Conditions

## EXISTING SITE DEVELOPMENT

The project site is currently occupied by multiple industrial tenants. Under existing conditions, the site contains two one-story industrial buildings, one two-story industrial building, and one three-story industrial building. Access to the site is provided via two full-movement driveways along River Road and a parking lot that opens directly to the road on Stone House Road. There are single family residences to the west and south-west of the site, commercial businesses to the south and east. To the north, there are commercial businesses and a train station. Train tracks run adjacent to the northern property line while the Passaic River runs along the western property line. An Aerial Map depicting the existing site conditions can be found in APPENDIX A.

## EXISTING TOPOGRAPHY

The high point of the subject site is at the northeast corner of the site abutting River Road and Division Avenue. River Road drains northwest towards current parking areas of adjacent lots, Division Avenue drains south towards Stone House Road and combines with the run-off of Stone House Road ultimately discharging into the Passaic River. On-site topography slopes toward low points within the site, collected by a stormwater system, and discharged to the Passaic River. Grades on the subject site average between $0 \%$ and $3.43 \%$. within the areas surrounding the buildings. However, steeper slopes are seen in the northern and southwestern corners of the subject site.

## PROJECT SITE SOILS

Soil mapping was obtained from the National Resource Conservation Service (NRCS) for the project site and immediate area. Generally, the project site is underlain with one major soil group: silt loam which occupies a majority of the site. Overall, the soils drain well, and runoff flows overland directly to the Passaic River. The table below provide a summary of soils for the project site:

## TABLE I: NRCS SOIL MApping Results

| Soil Unit <br> Code | Soil Description | Approximate <br> Project <br> Coverage | Drainage Class | Hydrologic <br> Soil Group |
| :---: | :---: | :---: | :---: | :---: |
| PeoC | Penn Channery Silt Loam, 8\% to <br> I5\% Slopes | $9.5 \%$ | Well drained | B |
| USPENB | Urban Land-Penn Complex, 0\% <br> to 8\% Slopes | $87.5 \%$ | Well drained | C |
| WATER | Passaic River | $3.0 \%$ | N/A | N/A |

*USPENB does not have a pre-determined hydraulic soil group due to high variability in the historic fill material utilized. As such, these soils are analyzed as HSG B under pre-existing conditions and HSG D under post-development conditions.

Additional information regarding the NRCS soil mapping can be found in APPENDIX B.

## EXISTING ENVIRONMENTAL INVENTORY

The project site is bounded by the Passaic River on the west with a NJDEP Restricted Area separating it from the proposed project site. The river is subject to a special flood hazard area as verified and delineated on FEMA Flood Insurance Map Panel \# 3403560005B. The limits of these areas are shown on the Critical Area Plan (Sheet C-I7) of the Preliminary and Final Major Site Plans prepared by Stonefield in conjunction with this Report.

### 3.0 Proposed Conditions

## PROPOSED SITE DEVELOPMENT

Under the proposed development plan, the project area will include a mixed-use family and commercial development. The proposed development includes the construction of fourteen 10 -unit multi-family residences, an I,800 SF community building, a 4,992 SF retail building, and supporting improvements inclusive of parking facilities, landscaping, utilities, site lighting, and stormwater management measures. The eastern portion of the site that is within the limit of disturbance is being collected via the proposed conveyance system on site and either being sent directly to the Passaic River or to the Municipal conveyance system via catch basins and HDPE pipes. Refer to APPENDIX A for a half-size Overall Site Plan depicting the proposed project improvements.

## PROPOSED TOPOGRAPHY

Project site topography and drainage patterns will generally remain similar to existing conditions; however, due to the need for more commercially friendly, ADA compliant grades ( $1.5 \%$ to $3 \%$ ) various retaining walls will be implemented through the project to make up for the change in grades.

## ANTICIPATED ENVIRONMENTAL INVENTORY IMPACTS

The proposed redevelopment will not disturb land within environmentally regulated areas (flood hazard area, riparian zone, freshwater wetland ditch, and freshwater wetland transition area). As such, permits and approvals will not be sought from the NJDEP to perform work within these areas.

### 4.0 Stormwater Management Methodology \& Parameters

## HYDROLOGIC METHODOLOGY

The analysis program "HydroCAD" Version 10.0 by HydroCAD Software Solutions was utilized to calculate and plot the runoff hydrographs. The program incorporates the time of concentration, C values, rainfall data, and project drainage areas to calculate the runoff characteristics. The existing and proposed drainage areas have been analyzed utilizing Intensity-Duration-Frequency data was obtained from NOAA for the project area; specifics of the rainfall distribution can be found in Appendix C. Additional key variables utilized in the analysis include:

TABLE 2: Hydrocad Design Variables

| Variable | Input | Variable | Input |
| :--- | :--- | :--- | :--- |
| Runoff Calculation Method | SCS TR-20 | NRCS Rainfall Frequency Data Set | Morris |
| Pervious/Impervious CN <br> Calculations | Separate | Storm Intervals (Year Events) | $2,10,25,100$ |
| Stage-Storage Relationship | Dynamic | Storm Duration | 24 Hours |
| Minimum time of concentration | 10 minutes | Storm Curve | NOAA D |

Additional information regarding the hydrologic calculations can be found in APPENIDX C.

## HYDRAULIC METHODOLOGY

The analysis program "HydraFlow Storm Sewers" Version 2018 by Autodesk was utilized to generate hydraulic grade lines through the proposed conveyance system model based on various pipe / junction losses and the runoff tributary to each inlet or discharge structure. Additional key variables utilized in the analysis include:

TABLE 3: Hydraflow Design Variables

| Variable | Input | Variable | Input |
| :--- | :--- | :--- | :--- |
| Runoff Calculation Method | Rational | Pipe Conveyance Method | Std. Step |
| C-value for impervious surfaces | 0.95 | Initial Hydraulic Grade Line | Normalized |
| C-value for pervious surfaces | 0.60 | Inlet Drainage Area Delineation | Surveyed |
| Minimum time of concentration | 10 minutes | Inlet Geometry \& Capacity | NJDOT Std. |

Additional information regarding the hydrologic calculations can be found in APPENDIX C.

### 5.0 Stormwater Analysis

## EXISTING DRAINAGE AREAS

Under existing conditions, the site is comprised of four drainage areas, and one Point of Interest (POI). The Point of Interest (POI-I) discharges to the Passaic River along the western property line. The site slopes from the northeastern corner of the site to the southwestern portion of the site. The existing slopes are not steep throughout the site with an average grade around $3.5 \%$. See below for a short summary of each area:

TABLE 4: Summary of Existing Drainage Areas

| Drainage <br> Area | Description | Area <br> Extents | Impervious <br> Area | Time of <br> Concentration |
| :---: | :--- | :---: | :---: | :---: |
| E-IA | Existing Drainage to Existing Conveyance <br> System (I8" Pipe) | 52,576 | 9,429 | $10^{*}$ |
| E-IB | Existing Drainage to Existing Conveyance <br> System (42" Pipe) | 141,668 | 141,668 | $10^{*}$ |
| E-IC | Existing Drainage to Existing Conveyance <br> System (I5" Pipe) | 80,227 | 78,544 | $10^{*}$ |
| E-ID | Existing Drainage to Municipal System | 52,934 | 43,589 | $10^{*}$ |
| POI (E-I) | Ultimate Point of Interest: Passaic River | 327,405 SF | 273,230 | N/A |

*The minimum time of concentration was utilized due to the high level of impervious coverage and proximity to the Passaic River.

All existing drainage areas were delineated based on field surveying data. Hydrologic calculations and parameters for each drainage area can be found in APPENDIX C; specific drainage area delineations and land cover can be found in APPENDIX D.

## PROPOSED DRAINAGE AREAS

Under proposed conditions the site is comprised of one (I) point of interest. POI-I is comprised of four subareas with all areas ultimately discharging to the Passaic River, consistent with existing drainage patterns. The subareas are either collected via proposed catch basins and sent directly to the Passaic River via existing conveyance pipes on-site or to the Municipal conveyance system within Stone House Road. Drainage area P-IA is discharged to the Passaic River via a connection to an existing catch basin on the northwest corner of the site with an outlet pipe 18 " in diameter. Drainage area P -IB is discharged to the Passaic River via a connection to an existing manhole located between Building \#2 and Building \#3 with an outlet pipe 42 " in diameter. Drainage area P-IC is discharged to the Passaic River via a connection to an existing catch basin located north of Building \#5 with an outlet pipe I5" in diameter. Drainage area P-ID is collected within the Municipal System in Stone House Road via sheet flow to various existing inlets which eventually discharge into the Passaic River. See below for a short summary of each area:

TABLE 5: Summary of Proposed Drainage Areas

| Drainage <br> Area | Description | Area <br> Extents | Impervious <br> Area | Time of <br> Concentration |
| :---: | :--- | :---: | :---: | :---: |
| P-IA | Proposed Drainage to Passaic River via <br> Proposed Conveyance System (I8" Pipe) | 50,878 | 20,909 | $10^{*}$ |
| P-IB | Proposed Drainage to Passaic River via <br> Proposed Conveyance System (42" Pipe) | 142,558 | 103,691 | $10^{*}$ |
| P-IC | Proposed Drainage to Passaic River via <br> Proposed Conveyance System (I5" Pipe) | 81,290 | 58,221 | $10^{*}$ |
| P-ID | Proposed Drainage Directly to Municipal <br> System | 52,679 | 23,598 | $10^{*}$ |
| POI (P-I) | Ultimate Point of Interest: Passaic River | 327,405 SF | 206,525 SF | N/A |

*The minimum time of concentration was utilized for all drainage areas due to the high level of impervious coverage / land disturbance and proximity to existing and proposed stormwater pipe conveyance system.

All proposed drainage areas were delineated based on the proposed grading design overlain on field survey data. Hydrologic calculations and parameters for each drainage area can be found in APPENDIX C; specific drainage area delineations and land cover can be found in APPENDIX D.

## STORMWATER MANAGEMENT DESIGN PARAMETERS

As the proposed improvement will disturb 7.79 acres of land, the project is defined as a "Major Development" as indicated in Town Ordinances and per NJDEP regulations. The proposed project will meet the stormwater quantity requirements by demonstrating that at no point in time does the post-development hydrograph or run-off volumes exceed the pre-development hydrograph or run-off volumes; as the analysis area is the same and impervious surfaces have been decreased, the post-construction quantities will at no point exceed the pre-development quantities. Groundwater recharge requirements do not apply as the site is located within the State Planning Area PA-I. Additionally, water quality requirements do not apply as the site is proposing to decrease impervious surfaces on-site by 2.32 acres per NJAC Section 7:8-5.5. See below for a summary of each design parameter and compliance requirements:

TABLE 6: Stormwater Management Design Target Summary

| Design Parameter | Design Target for Compliance |
| :--- | :--- |
| Stormwater Runoff <br> Quantity | Demonstrate through hydrologic and hydraulic analysis that for stormwater leaving <br> the site, post-construction runoff hydrographs for the 2, IO, and 100-year storm <br> events do not exceed, at any point in time, the pre-construction runoff hydrographs <br> for the same storm events. |
| Groundwater <br> Recharge | The project is exempt from groundwater recharge requirements as the project site <br> is located within State Planning Area PA-I (Metropolitan). |
| Water Quality | The project is exempt from water quality requirements as the project site is <br> proposing to decrease impervious surfaces on-site per NJAC Section 7:8-5.5 |

## STORMWATER RUNOFF QUANTITY

Runoff is controlled through the implementation of the reduction of impervious area on site. The tables below summarize the various drainage areas in relation to flow rates and runoff volume during regulatory storm events:
table 7: Summary of Existing Drainage Area Flow Rates \& Volumes

| Drainage Area | 2-Year Flow Rate | I0-Year Flow Rate | I00-Year Flow Rate |
| :---: | :---: | :---: | :---: |
| E-IA | 3.14 CFS | 4.89 CFS | 8.29 CFS |
| E-IB | 9.18 CFS | 13.91 CFS | 22.97 CFS |
| E-IC | 5.16 CFS | 7.84 CFS | 12.98 CFS |
| E-ID | 3.16 CFS CFS | 4.93 CFS | 8.35 CFS |
| POI (E-I) | 20.64 CFS | 31.57 CFS | 52.59 CFS |

TABLE 8: Summary of Proposed Drainage Area Flow Rates \& Volumes

| Drainage Area | 2-Year Flow Rate | I0-Year Flow Rate | I00-Year Flow Rate |
| :---: | :---: | :---: | :---: |
| P-IA | 2.44 CFS | 4.15 CFS | 7.50 CFS |
| P-IB | 8.13 CFS | 12.89 CFS | 22.14 CFS |
| P-IC | 4.61 CFS | 7.33 CFS | 12.60 CFS |
| P-ID | 2.59 CFS | 4.35 CFS | 7.81 CFS |
| POI (P-I) | 17.77 CFS | 28.71 CFS | 50.05 CFS |

Under post-development conditions the runoff flow rates and volumes are reduced to the undetained drainage areas including Stone Hill Road (E-ID/P-ID). The diverted runoff from these areas are collected in the on-site
stormwater management system ( $\mathrm{E}-\mathrm{IA} / \mathrm{P}-\mathrm{IA}, \mathrm{E}-\mathrm{IB} / \mathrm{P}-\mathrm{IB}$, and $\mathrm{E}-\mathrm{IC} / \mathrm{P}-\mathrm{IC}$ ) and conveyed directly to discharge into the Passaic River. The table below outlines the regulatory compliance parameters for runoff quantity on the project site:

TABLE 9: Stormwater Runoff Quantity Compliance Summary at Point of Interest (E-I / P-I)

| Rainfall Event | Existing <br> Flow Rate | Required \% <br> Reduction | Required <br> Flow Rate | Proposed <br> Flow Rate | Proposed \% <br> Reduction |
| :--- | :---: | :---: | :---: | :---: | :---: |
| 2-Year Storm | 20.64 CFS | N/A | 20.39 CFS | 17.77 CFS | $13.91 \%$ |
| I0-Year Storm | 31.57 CFS | N/A | 31.25 CFS | 28.71 CFS | $9.06 \%$ |
| I00-Year <br> Storm | 52.59 CFS | N/A | 52.19 CFS | 50.05 CFS | $4.83 \%$ |

The reduction of impervious area on site decreases the flow rate as to ensure that no adverse impacts are anticipated downstream of the project site. Detailed hydrologic calculations for each drainage area can be found in APPENDIX C.

## GROUNDWATER RECHARGE

As indicated in the Township Ordinances and NJAC 7:8-5.4, the project site is exempt from groundwater recharge requirements as the site is located within the Metropolitan Planning Area (PA-I) per the State Plan Policy Map and thus qualifies as an Urban Redevelopment Area (which is exempt from groundwater recharge requirements).

## STORMWATER PIPE CONVEYANCE SYSTEM

The on-site stormwater conveyance system has been sized for the 25 -year storm event and is able to safely convey runoff to the proposed stormwater management facilities without overflow or bypass. Detailed hydraulic calculations for the conveyance system can be found in APPENDIX C. See below for a table summarizing the various drainage areas during the 25 -year storm event:

TABLE I0: SUMMARY OF 25-YEAR STORM (FOR DRAINAGE DIRECT TO PUBLIC ROW OR TRIBUTARY)

| Tributary Area | Existing Flow Rate | Proposed Flow Rate | Flow Rate Difference | Existing Volume | Proposed Volume | Volume Difference |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Drainage Direct to Passaic River (E-IA/P-IA, $\mathrm{E}-\mathrm{IB} / \mathrm{P}-\mathrm{IB}$, and E-IC/P-IC) | 32.93 CFS | 30.74 CFS | -2.19 CFS | 137,739 CF | I24,277 CF | -13,462 CF |
| Drainage Direct to Municipal System (E-ID / P-ID) | 6.15 CFS | 5.58 CFS | -0.57 CFS | 25,348 CF | 21,909 CF | -3,439 CF |
| Overall Drainage to Passaic River (E-I/P-I) | 39.08 CFS | 36.33 CFS | -2.75 CFS | 163,087 CF | 146, I86 CF | -16,901 CF |

The runoff flow rates and volumes that directly tributary to the existing Township stormwater pipe conveyance systems within Stone House Road (E-ID/P-ID) are significantly reduced under proposed conditions. As such, no adverse impacts to the adjacent existing stormwater infrastructure is anticipated. Additionally, the volume of stormwater runoff is proposed to decrease overall to the ultimate point of interest (due to the decrease in impervious coverage on-site) the flow rates are significantly reduced and the proposed stormwater management and soil erosion features ensure that runoff entering the Passaic River is safely conveyed so as to not cause any adverse impacts further downstream.

### 6.0 Stormwater Facility Operations \& Maintenance

A Stormwater Operations \& Maintenance Manual will be submitted for approval to the Morris County Soil Conservation District prior to the start construction. Any required easements or covenants associated with the stormwater improvements will be recorded prior to the start of construction.

### 7.0 Erosion \& Sediment Control

A Soil Erosion \& Sediment Control Plan has been prepared in accordance with the latest edition of the Standards for Soil Erosion and Sediment Control in New Jersey. Proposed temporary measures during construction include silt fencing, stabilized construction entrances, inlet filters, and cover for soil stabilization. No land disturbance will occur until a permit has been obtained from the Morris County Soil Conservation District.

### 8.0 CONCLUSIONS

The proposed project complies with all applicable stormwater management regulations and standards. As such, the project is not anticipated to have any adverse impacts or neighboring properties, downstream watercourses, or conveyance systems within the watershed.

### 9.0 REFERENCES

I. New Jersey Administrative Code Title 7, Chapter 8 Stormwater Management, last amended June 20, 2016 https://www.nj.gov/dep/rules/rules/njac7_8.pdf
2. New Jersey Stormwater Best Management Practices Manual, last revised November 2018 https://www.njstormwater.org/bmp_manual2.htm
3. Township of Long Hill Land Use Ordinance, last amended May 31, 2019 https://clerkshq.com/LongHill-ni

# APPENDIX A Project Figures 

## INVENTORY

Figure I: USGS Location Map
Figure 2: Aerial Map
Figure 3: Tax \& Zoning Map
Figure 4: FEMA MAP
Figure 5: Site Plan (Not to Scale)



 INSURANCE RATE MAP

GRAPHIC SCALE IN FEET


## $\mathrm{I}^{\prime \prime}=1000^{\prime}$

| DRAWN BY: |  |
| :--- | :--- |
|  | BVT |
| CHECKED BY: | SMO |
| DATE: | $06 / 11 / 2019$ |
| SCALE: | $\mathrm{I}^{\prime \prime}=1000^{\prime}$ |
| PROJECT ID: | T-17298 |

##  <br> STONEFIELD engineering \& design

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## APPENDIX B NRCS SOILS REPORT

United States Department of Agriculture


Natural
Resources
Conservation
Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Morris County, New Jersey



## MAP LEGEND

| Area of Interest (AOI) |  |
| :--- | :--- |
| $\square$ | Area of Interest (AOI) |
| Soils |  |
| $\square$ | Soil Map Unit Polygons |
| $\square$ | Soil Map Unit Lines |
| $\square$ | Soil Map Unit Points |

Special Point Features
(c) Blowout

B Borrow Pit
次 Clay Spot
$\diamond$ Closed Depression
Gravel Pit
$\therefore \quad$ Gravelly Spot
(4) Landfill
A. Lava Flow

Marsh or swamp
\& Mine or Quarry
(-) Miscellaneous Water

- Perennial Water
- Rock Outcrop
+ Saline Spot
$\because \quad$ Sandy Spot
을 Severely Eroded Spot
- Sinkhole

3) Slide or Slip
(6) Sodic Spot

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.
Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Morris County, New Jersey Survey Area Data: Version 13, Sep 13, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 31, 2009—Feb 26, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background magery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# Map Unit Legend 

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
| :--- | :--- | ---: | ---: |
| PeoC | Penn channery silt loam, 8 to <br> 15 percent slopes | 1.1 |  |
| USPENB | Urban land-Penn complex, 0 to <br> 8 percent slopes | 10.7 |  |
| WATER | Water | $0.3 \%$ |  |
| Totals for Area of Interest |  | $\mathbf{1 2 . 1}$ |  |

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.
A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.
Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.
The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The
delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a soil series. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into soil phases. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A complex consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An undifferentiated group is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include miscellaneous areas. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Morris County, New Jersey

## PeoC—Penn channery silt loam, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 2 tt 83
Elevation: 250 to 800 feet
Mean annual precipitation: 38 to 53 inches
Mean annual air temperature: 43 to 57 degrees F
Frost-free period: 170 to 240 days
Farmland classification: Farmland of statewide importance

## Map Unit Composition

Penn and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Penn

## Setting

Landform: Hills
Landform position (two-dimensional): Shoulder, backslope, summit Landform position (three-dimensional): Interfluve, side slope Down-slope shape: Convex
Across-slope shape: Linear
Parent material: Residuum weathered from shale and siltstone

## Typical profile

Ap - 0 to 10 inches: channery silt loam
Bt1-10 to 15 inches: channery silt loam
Bt2 - 15 to 19 inches: channery silt loam
Bt3 - 19 to 22 inches: channery loam
C-22 to 28 inches: very channery loam
$R$ - 28 to 38 inches: bedrock
Properties and qualities
Slope: 8 to 15 percent
Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high ( 0.14 to $1.28 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 4.6 inches)
Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: B
Hydric soil rating: No

## Minor Components

## Klinesville

Percent of map unit: 10 percent
Landform: Hills
Landform position (two-dimensional): Backslope, shoulder Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

## Croton

Percent of map unit: 5 percent
Landform: Depressions
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope
Down-slope shape: Linear, concave
Across-slope shape: Linear, concave
Hydric soil rating: Yes

## Readington

Percent of map unit: 5 percent
Landform: Depressions
Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Base slope
Down-slope shape: Linear, concave
Across-slope shape: Linear, concave
Hydric soil rating: No

## USPENB—Urban land-Penn complex, 0 to 8 percent slopes

## Map Unit Setting

National map unit symbol: 13q0b
Elevation: 250 to 1,300 feet
Mean annual precipitation: 30 to 64 inches
Mean annual air temperature: 46 to 79 degrees F
Frost-free period: 131 to 178 days
Farmland classification: Not prime farmland

## Map Unit Composition

Urban land: 55 percent
Penn and similar soils: 35 percent
Minor components: 10 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Urban Land

Setting
Landform: Hills
Down-slope shape: Linear, convex

Across-slope shape: Linear
Parent material: Surface covered by pavement, concrete, buildings, and other structures underlain by disturbed and natural soil material

## Typical profile

C-0 to 60 inches: variable

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8s
Hydric soil rating: Unranked

## Description of Penn

## Setting

Landform: Hills
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Fine-loamy residuum weathered from acid reddish shale, siltstone, and fine-grain sandstone

## Typical profile

A-0 to 8 inches: channery silt loam
$B A-8$ to 14 inches: channery silt loam
$B-14$ to 24 inches: channery silt loam
BC - 24 to 30 inches: channery silt loam
C-30 to 36 inches: very channery silt loam
$R-36$ to 80 inches: weathered bedrock

## Properties and qualities

Slope: 0 to 6 percent
Depth to restrictive feature: 20 to 39 inches to lithic bedrock
Natural drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high ( 0.20 to $2.00 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 5.8 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2 e
Hydrologic Soil Group: C
Hydric soil rating: No

## Minor Components

## Klinesville

Percent of map unit: 5 percent
Landform: Hills
Landform position (two-dimensional): Shoulder
Down-slope shape: Linear
Across-slope shape: Convex
Hydric soil rating: No

# Custom Soil Resource Report 

## Reaville

Percent of map unit: 5 percent
Landform: Interfluves
Down-slope shape: Convex
Across-slope shape: Linear
Hydric soil rating: No

## WATER-Water

## Map Unit Setting

National map unit symbol: b0p9
Mean annual precipitation: 30 to 64 inches
Mean annual air temperature: 46 to 79 degrees F
Frost-free period: 131 to 178 days
Farmland classification: Not prime farmland

## Map Unit Composition

Water: 100 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

# APPENDIX C Hydrologic \& Hydraulic CALCULATIONS 

## INVENTORY

C-I: NOAA Rainfall Frequency Data
C-2: Hydrocad Node Schematic Diagram
C-3: HydroCAD Hydrologic Calculations
C-4: HydraFlow Pipe Network Summary Chart C-5: HydraFlow Hydraulic Pipe Analysis

NOAA Atlas 14, Volume 2, Version 3
Location name: Millington, New Jersey, USA*
Latitude: $\mathbf{4 0 . 6 7 1 8}^{\circ}$, Longitude: $\mathbf{- 7 4 . 5 2 4 5 ^ { \circ }}$
Elevation: 249.16 ft** $^{*}$

* source: ESRI Maps
** source: USGS



## POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M. Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland
PF tabular I PF_graphical | Maps \& aerials

## PF tabular

| PDS-based point precipitation frequency estimates with 90\% confidence intervals (in inches) ${ }^{1}$ |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Duration | Average recurrence interval (years) |  |  |  |  |  |  |  |  |  |
|  | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 |
| 5-min | (0.305-0.369) |  | (0.428-0.519) | $\mid(0.475-0.577)$ | $(0.531-0.646)$ | (0.569-0.695) | $\mid(0.605-0.743)$ | $\begin{gathered} \mathbf{0 . 7 1 5} \\ (0.636-0.785) \\ \hline \end{gathered}$ | $(0.674-0.840)$ |  |
| 10-1 | $\begin{array}{r} \mathbf{0 . 5 3 4} \\ (0.486-0.5 \end{array}$ | $637$ | 0) | $.83$ |  |  | $1.07$ | $1.13$ | $.21$ | $1.25$ |
| 15 | $\begin{array}{r} \mathbf{0 . 6 6 7} \\ (0.608-0.7 \end{array}$ | $\begin{array}{r} 0.800 \\ (0.729-0.8 \end{array}$ |  | $\begin{array}{c\|} \hline 1.06 \\ (0.959-1.16) \\ \hline \end{array}$ |  |  |  | $\begin{gathered} 1.43 \\ (1.27-1.57) \end{gathered}$ | $\begin{gathered} 1.52 \\ (1.34-1.67) \end{gathered}$ | $\begin{gathered} 1.58 \\ (1.38-1.74) \\ \hline \end{gathered}$ |
| 30-min |  | $10$ | $1 .$ | $53$ |  |  |  | $\begin{aligned} & .22 \\ & 7-2.44) \\ & \hline \end{aligned}$ | $\begin{aligned} & .41 \\ & 3-2.65) \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 2.54 \\ & 23-2.81) \\ & \hline \end{aligned}$ |
| 60-min | $\begin{array}{r} 1.14 \\ (1.04-1.2 \\ \hline \end{array}$ |  | $\begin{gathered} 1.73 \\ (1.58-1.91) \end{gathered}$ | $\begin{gathered} 1.99 \\ (1.81-2.19) \end{gathered}$ |  |  |  |  |  |  |
| 2-hr | $(1.26-1.54)$ | $\begin{gathered} 1.70 \\ (1.54-1.88) \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 . 1 5} \\ (1.95-2.38) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 . 5 0} \\ (2.26-2.77) \end{gathered}$ |  | $\begin{gathered} \hline 3.39 \\ (3.02-3.74) \\ \hline \end{gathered}$ | $\begin{gathered} 3.80 \\ (3.37-4.19) \\ \hline \end{gathered}$ | $\begin{gathered} 4.23 \\ (3.72-4.66) \end{gathered}$ | $\begin{gathered} \hline 4.83 \\ (4.20-5.34) \end{gathered}$ | $\begin{gathered} 5.30 \\ (4.57-5.87) \end{gathered}$ |
| 3-hr | $(1.41-1.73)$ | $\begin{gathered} 1.90 \\ (1.72-2.11) \end{gathered}$ | $\begin{gathered} 2.41 \\ (2.18-2.68) \end{gathered}$ | $\begin{gathered} \mathbf{2 . 8 1} \\ (2.53-3.11) \end{gathered}$ | $(3.01-3.71)$ | $\begin{gathered} 3.80 \\ (3.40-4.20) \\ \hline \end{gathered}$ | $\begin{gathered} 4.26 \\ (3.78-4.71) \end{gathered}$ |  |  |  |
| 6-hr | $(1.82-2.23)$ | $\begin{gathered} \mathbf{2 . 4 3} \\ (2.21-2.70) \\ \hline \end{gathered}$ | $(2.79-3.41)$ | $(3.26-3.98)$ | $(3.90-4.80)$ | (4.44-5.49) | (4.99-6.23) | $(5.58-7.01)$ | $(6.39-8.17)$ | (7.05-9.13) |
| 12-hr | (2.26-2.7 | 2.74-3.3 | (3.48-4.2 | (4.10-5.0 | (4.98-6.1 | (5.72-7.0 |  |  | (8.55-11.0) |  |
| 24-hr | (2.62-3 | 3.17-3.72 | (4.03- | (4.75- | (5.81-6.87 | (6.69-7.97) | (7.65-9.19) | (8.68-10.6) | $\begin{gathered} 11.6 \\ (10.2-12.6) \end{gathered}$ | $\begin{gathered} 13.2 \\ (11.4-14.4) \end{gathered}$ |
| 2-day | $(3.06-3.64)$ | (3.70-4.40) | $\begin{gathered} 5.11 \\ (4.69-5.59) \end{gathered}$ | $\begin{gathered} 6.01 \\ (5.51-6.57) \end{gathered}$ | (6.67-8.00) | (7.64-9.20) | $(8.66-10.5)$ | (9.74-12.0) | $(11.3-14.1)$ | (12.5-15.9) |
| 3-day | $(3.23-3.82)$ | $(3.90-4.61)$ | (4.93-5.84) | $\begin{gathered} 6.28 \\ (5.77-6.84) \\ \hline \end{gathered}$ | (6.96-8.30) | (7.95-9.52) | $(8.98-10.8)$ | (10.1-12.3) | $(11.6-14.4)$ | $\begin{gathered} 14.7 \\ (12.8-16.2) \end{gathered}$ |
| 4-d | $(3.40-4.00)$ | $\begin{gathered} \hline 4.44 \\ (4.11-4.83) \end{gathered}$ | $\begin{gathered} 5.61 \\ (5.17-6.10) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{6 . 5 6} \\ (6.03-7.12) \end{gathered}$ | $(7.26-8.60)$ | (8.25-9.84) | $(9.29-11.2)$ | $\begin{gathered} 11.6 \\ (10.4-12.6) \\ \hline \end{gathered}$ | $\begin{gathered} 13.4 \\ (11.9-14.7) \end{gathered}$ | (13.1-16.5) |
| 7-day | (4.03-4.70) | $\begin{gathered} 5.21 \\ (4.84-5.64) \end{gathered}$ | $\begin{gathered} \mathbf{6 . 4 6} \\ (5.99-6.99) \\ \hline \end{gathered}$ | $\begin{gathered} 7.49 \\ (6.92-8.10) \\ \hline \end{gathered}$ | (8.24-9.69) | (9.31-11.0) | (10.4-12.4) | $(11.6-14.0)$ | $(13.2-16.2)$ | (14.5-18.0) |
| 10-day | (4.64-5.36) | $\begin{gathered} 5.95 \\ (5.55-6.40) \end{gathered}$ | $\begin{gathered} 7.27 \\ (6.76-7.82) \end{gathered}$ | (7.75-8.98) | $\begin{gathered} 9.86 \\ (9.11-10.6) \end{gathered}$ | (10.2-11.9) | $(11.3-13.4)$ | (12.5-14.9) | $(14.0-17.0)$ | (15.3-18.7) |
| 20-da | (6.32-7.17) | (7.49-8.51) | (8.93-10.1) | $(10.1-11.4)$ | (11.5-13.2) | (12.7-14.5) | (13.8-15.9) | (15.0-17.4) | (16.4-19.3) | $(17.5-20.8)$ |
| 30-da | (7.92-8.85) | $(9.35-10.4)$ | (10.9-12.2) | $\begin{gathered} \hline 12.8 \\ (12.1-13.5) \\ \hline \end{gathered}$ | (13.6-15.3) | (14.8-16.6) | (15.9-17.9) | (16.9-19.3) | (18.3-21.0) | $(19.2-22.2)$ |
| 45-day | (10.1-11.2) | (11.9-13.2) | $\begin{gathered} 14.4 \\ (13.7-15.2) \end{gathered}$ | $\begin{gathered} 15.8 \\ (15.0-16.7) \\ \hline \end{gathered}$ | (16.7-18.6) | $(18.0-20.1)$ | (19.2-21.5) | $(20.3-22.8)$ | $(21.7-24.6)$ | $(22.6-25.8)$ |
| 60-day | $\begin{gathered} \hline 12.8 \\ (12.1-13.4) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathbf{1 5 . 0} \\ (14.2-15.7) \end{gathered}$ | $\begin{gathered} 17.1 \\ (16.3-17.9) \end{gathered}$ | $\begin{gathered} \hline 18.7 \\ (17.8-19.6) \end{gathered}$ | $\begin{gathered} \hline \mathbf{2 0 . 6} \\ (19.6-21.7) \end{gathered}$ | $\begin{gathered} 22.1 \\ (20.9-23.2) \\ \hline \end{gathered}$ | $\begin{gathered} \mathbf{2 3 . 4} \\ (22.2-24.7) \end{gathered}$ | $\begin{gathered} \mathbf{2 4 . 7} \\ (23.3-26.0) \end{gathered}$ | $\begin{gathered} \mathbf{2 6 . 2} \\ (24.6-27.7) \end{gathered}$ | $\begin{gathered} \mathbf{2 7 . 2} \\ (25.6-28.9) \end{gathered}$ |

[^0]
## PF graphical



Time span=0.00-30.00 hrs, $\mathrm{dt}=0.01 \mathrm{hrs}, 3001$ points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentE-1B: Existing Drainage Runoff Area=141,668 sf $100.00 \%$ Impervious Runoff Depth=3.19" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=0 / 98$ Runoff=9.18 cfs $37,621 \mathrm{cf}$

SubcatchmentE-1C: Existing Drainageto Runoff Area=80,227 sf $97.90 \%$ Impervious Runoff Depth=3.16" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=83 / 98$ Runoff=$=5.16 \mathrm{cfs} 21,109 \mathrm{cf}$

SubcatchmentE-1D: Existing Drainage to Runoff Area=52,934 sf $82.35 \%$ Impervious Runoff Depth $=2.90$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=3.16 \mathrm{cfs} 12,801 \mathrm{cf}$

SubcatchmentP-1A: Proposed Drainage to Runoff Area=50,878 sf $41.10 \%$ Impervious Runoff Depth=2.24" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff=$=2.44 \mathrm{cfs} 9,481 \mathrm{cf}$

SubcatchmentP-1B: Proposed Drainage Runoff Area=142,558 sf 72.74\% Impervious Runoff Depth=2.75" Tc=10.0 min CN=80/98 Runoff=8.13 cfs $32,631 \mathrm{cf}$

SubcatchmentP-1C: Proposed Drainage to Runoff Area=81,290 sf 71.62\% Impervious Runoff Depth=2.73" $\mathrm{Tc}=10.0 \mathrm{~min} \quad \mathrm{CN}=80 / 98$ Runoff $=4.61 \mathrm{cfs} 18,485 \mathrm{cf}$

SubcatchmentP-1D: Proposed Drainage to Runoff Area=52,679 sf $44.80 \%$ Impervious Runoff Depth=2.30" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=2.59 \mathrm{cfs} 10,079 \mathrm{cf}$

Link E-1: Existing Drainage to Passaic River Inflow=20.64 cfs 84,226 cf Primary $=20.64$ cfs 84,226 cf

Link P-1: Proposed Drainage to Passaic River

Total Runoff Area $=654,810$ sf Runoff Volume $=154,903$ cf Average Runoff Depth $=2.84$ $\mathbf{2 1 . 6 0 \%}$ Pervious $=141,443 \mathrm{sf} \quad 78.40 \%$ Impervious $=513,367 \mathrm{sf}$

Summary for Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)
Runoff $=3.14$ cfs @ 12.17 hrs, Volume $=12,694 \mathrm{cf}$, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 43,147 | 98 | Impervious Areas |
| 9,429 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 52,576 | 95 | Weighted Average |
| 9,429 | 80 | 17.93\% Pervious Area |
| 43,147 | 98 | $82.07 \%$ Impervious Area |

\(\left.$$
\begin{array}{rrrl}\begin{array}{r}\text { Tc } \\
(\mathrm{min})\end{array} & \begin{array}{r}\text { Length } \\
(\mathrm{feet})\end{array} & \begin{array}{r}\text { Slope } \\
(\mathrm{ft} / \mathrm{ft})\end{array} & \begin{array}{r}\text { Velocity } \\
(\mathrm{ft} / \mathrm{sec})\end{array}\end{array}
$$ \begin{array}{r}Capacity <br>

(\mathrm{cfs})\end{array}\right)\) Description | Direct Entry, Direct |
| :--- |

## Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)



Summary for Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)
Runoff $=\quad 9.18$ cfs @ 12.17 hrs, Volume $=\quad 37,621 \mathrm{cf}$, Depth= 3.19"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

|  | Area (sf) | CN | Description |
| :--- | ---: | ---: | :--- |
| * | 141,668 | 98 | Impervious Areas |
| 141,668 | 98 | $100.00 \%$ Impervious Area |  |


| Tc | Length <br> $(\mathrm{min})$ | Slope <br> (feet) | Velocity <br> (ft/ft) | Capacity <br> (ft/sec) |
| ---: | ---: | ---: | ---: | :--- |
| 10.0 |  |  | Description |  |
| $(\mathrm{cfs})$ |  |  |  |  |

## Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)

$$
\text { Runoff }=5.16 \text { cfs @ } 12.17 \text { hrs, Volume= } \quad 21,109 \mathrm{cf} \text {, Depth= } 3.1^{\prime \prime}
$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 78,544 | 98 Im | Impervious Areas |  |  |
|  | 353 | 96 | Gravel surface, HSG D |  |  |
|  | 1,330 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 80,227 | 98 V | Weighted Average |  |  |
|  | 1,683 | 83 | 2.10\% Pervious Area |  |  |
|  | 78,544 | 989 | 97.90\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | $\begin{array}{r} \text { c } \begin{array}{r} \text { Length } \\ \text { (feet) } \\ \hline \end{array} \\ \hline \end{array}$ | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 10.0 |  |  |  |  | Direct Entry |

Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)



Summary for Subcatchment E-1D: Existing Drainage to Municipal System
Runoff $=3.16$ cfs @ 12.17 hrs, Volume $=12,801 \mathrm{cf}$, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 43,589 | 98 | Impervious Areas |  |  |
|  | 9,345 | 80 | >75\% Grass cover, Good, HSG D |  |  |
|  | 52,934 | 95 Weighted Average |  |  |  |
|  | 9,345 | 80 | 17.65\% Pervious Area |  |  |
|  | 43,589 | 98 | 82.35\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 10.0 |  |  |  |  | Direct Entry |

Subcatchment E-1D: Existing Drainage to Municipal System


## Summary for Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)

Runoff $=\quad 2.44$ cfs @ 12.17 hrs, Volume= $\quad 9,481 \mathrm{cf}$, Depth= 2.24"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 20,909 | 98 | Impervious Areas |
| 29,969 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 50,878 | 87 | Weighted Average |
| 29,969 | 80 | $58.90 \%$ Pervious Area |
| 20,909 | 98 | $41.10 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $($ feet $)$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)


## Summary for Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)

Runoff $=8.13$ cfs @ 12.17 hrs, Volume $=\quad 32,631 \mathrm{cf}$, Depth= $2.75{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

|  | Area (sf) | CN | Description |
| :---: | :---: | :---: | :---: |
| * | 103,691 | 98 | Impervious Areas |
|  | 38,867 | 80 | >75\% Grass cover, Good, HSG D |
|  | 142,558 | 93 | Weighted Average |
|  | 38,867 | 80 | 27.26\% Pervious Area |
|  | 103,691 | 98 | 72.74\% Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)


## Summary for Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)

Runoff $=\quad 4.61$ cfs @ 12.17 hrs, Volume $=\quad 18,485$ cf, Depth= 2.73"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

|  | Area (sf) | CN |
| ---: | ---: | :--- | Description $\quad$| * | 58,221 | 98 |
| :--- | ---: | :--- |
| Impervious Areas |  |  |
| 23,069 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 81,290 | 93 | Weighted Average |
| 23,069 | 80 | $28.38 \%$ Pervious Area |
| 58,221 | 98 | $71.62 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ |
| ---: | ---: | | Slope |
| ---: |
| $(\mathrm{ft} / \mathrm{ft})$ | | Velocity |
| ---: |
| $(\mathrm{ft} / \mathrm{sec})$ | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, Direct |
| :--- |

Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)


Summary for Subcatchment P-1D: Proposed Drainage to Municipal System
Runoff $=\quad 2.59$ cfs @ 12.17 hrs, Volume= $\quad 10,079 \mathrm{cf}$, Depth= 2.30"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 2-Year Rainfall=3.42"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 23,598 | 98 I | Impervious Areas <br> $>75 \%$ Grass cover, Good, HSG D |  |  |
|  | 29,081 | $80>$ |  |  |  |
|  | 52,679 | 88 | Weighted Average 55.20\% Pervious Area 44.80\% Impervious Area |  |  |
|  | 29,081 | 80 |  |  |  |
|  | 23,598 | 98 |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 10.0 |  |  |  |  | Direct Entry |

## Subcatchment P-1D: Proposed Drainage to Municipal System



Summary for Link E-1: Existing Drainage to Passaic River
Inflow Area = 327,405 sf, 93.75\% Impervious, Inflow Depth = 3.09" for 2-Year event
Inflow = 20.64 cfs @ 12.17 hrs, Volume=

84,226 cf
Primary $=20.64$ cfs @ 12.17 hrs , Volume $=\quad 84,226 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.0 min
Primary outflow $=$ Inflow, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$

## Link E-1: Existing Drainage to Passaic River



## Summary for Link P-1: Proposed Drainage to Passaic River

Inflow Area $=\quad 327,405$ sf, 63.05\% Impervious, Inflow Depth $=2.59$ " for 2-Year event
Inflow $=17.77$ cfs @ 12.17 hrs, Volume $=\quad 70,677$ cf
Primary = $\quad 17.77$ cfs @ 12.17 hrs , Volume $=\quad 70,677 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.0 min

Primary outflow $=$ Inflow, Time Span $=0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Link P-1: Proposed Drainage to Passaic River


Time span=0.00-30.00 hrs, $\mathrm{dt}=0.01 \mathrm{hrs}, 3001$ points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method
SubcatchmentE-1A: Existing Drainage to Runoff Area=52,576 sf $82.07 \%$ Impervious Runoff Depth=4.57"
Tc=10.0 $\min \quad$ CN $=80 / 98$ Runoff $=4.89 \mathrm{cfs} 20,041 \mathrm{cf}$
SubcatchmentE-1B: Existing Drainage Runoff Area=141,668 sf $100.00 \%$ Impervious Runoff Depth=4.91" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=0 / 98$ Runoff=13.91 cfs 58,001 cf

SubcatchmentE-1C: Existing Drainageto Runoff Area=80,227 sf $97.90 \%$ Impervious Runoff Depth=4.88" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=83 / 98$ Runoff=7.84 cfs $32,621 \mathrm{cf}$

SubcatchmentE-1D: Existing Drainage to Runoff Area=52,934 sf $82.35 \%$ Impervious Runoff Depth $=4.58$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=4.93 \mathrm{cfs} 20,201 \mathrm{cf}$

SubcatchmentP-1A: Proposed Drainage to Runoff Area=50,878 sf $41.10 \%$ Impervious Runoff Depth $=3.80$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff=4.15 cfs 16,113 cf

SubcatchmentP-1B: Proposed Drainage Runoff Area=142,558 sf $72.74 \%$ Impervious Runoff Depth=4.40" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=12.89 \mathrm{cfs} 52,247 \mathrm{cf}$

SubcatchmentP-1C: Proposed Drainage to Runoff Area=81,290 sf $71.62 \%$ Impervious Runoff Depth=4.38" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=7.33 \mathrm{cfs} 29,650 \mathrm{cf}$

SubcatchmentP-1D: Proposed Drainage to Runoff Area=52,679 sf $44.80 \%$ Impervious Runoff Depth $=3.87$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=4.35 \mathrm{cfs} 16,990 \mathrm{cf}$

Link E-1: Existing Drainage to Passaic River Inflow=31.57 cfs 130,864 cf Primary=31.57 cfs 130,864 cf

Link P-1: Proposed Drainage to Passaic River
Inflow=28.71 cfs 115,000 cf Primary=28.71 cfs 115,000 cf

[^1]Summary for Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)
Runoff $=\quad 4.89$ cfs @ 12.17 hrs, Volume $=\quad 20,041 \mathrm{cf}$, Depth= 4.57"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 43,147 | 98 In | Impervious Areas |  |  |
|  | 9,429 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 52,576 | 95 W | Weighted Average |  |  |
|  | 9,429 | 801 | 17.93\% Pervious Area |  |  |
|  | 43,147 | 988 | 82.07\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 10.0 |  |  |  |  | Direct Entry |

Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)


Summary for Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)
Runoff $=13.91$ cfs @ 12.17 hrs, Volume= $58,001 \mathrm{cf}$, Depth= 4.91"
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

|  | Area (sf) | CN | Description |
| :--- | ---: | ---: | :--- |
| 141,668 | 98 | Impervious Areas |  |
|  | 98 | $100.00 \%$ Impervious Area |  |


| Tc | Length <br> $(\mathrm{min})$ | Slope <br> (feet) | Velocity <br> (ft/ft) | Capacity <br> (ft/sec) |
| ---: | ---: | ---: | ---: | :--- |
| 10.0 |  |  | Description |  |
| $(\mathrm{cfs})$ |  |  |  |  |

## Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)

Runoff $=7.84$ cfs @ 12.17 hrs, Volume $=\quad 32,621 \mathrm{cf}$, Depth= 4.88"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 78,544 | 98 Im | Impervious Areas |  |  |
|  | 353 | 96 G | Gravel surface, HSG D |  |  |
|  | 1,330 | $80>$ | >75\% Grass cover, Good, HSG D |  |  |
|  | 80,227 | 98 V | Weighted Average |  |  |
|  | 1,683 | 832 | 2.10\% Pervious Area |  |  |
|  | 78,544 | 989 | 97.90\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 10.0 |  |  |  |  | Direct Entry |

Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)


Summary for Subcatchment E-1D: Existing Drainage to Municipal System
Runoff $=\quad 4.93$ cfs @ 12.17 hrs, Volume= $20,201 \mathrm{cf}$, Depth= 4.58"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 43,589 | 98 | Impervious Areas |  |  |
|  | 9,345 | 80 | >75\% Grass cover, Good, HSG D |  |  |
|  | 52,934 | 95 | Weighted Average <br> 17.65\% Pervious Area <br> 82.35\% Impervious Area |  |  |
|  | 9,345 | 80 |  |  |  |
|  | 43,589 | 98 |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 10.0 |  |  |  |  | Direct Entry |

Subcatchment E-1D: Existing Drainage to Municipal System


## Summary for Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)

Runoff $=\quad 4.15$ cfs @ 12.17 hrs, Volume $=16,113 \mathrm{cf}$, Depth= $3.80^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| * | 20,909 | 98 |
| Impervious Areas |  |  |
| 29,969 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 50,878 | 87 | Weighted Average |
| 29,969 | 80 | $58.90 \%$ Pervious Area |
| 20,909 | 98 | $41.10 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $($ feet $)$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)


## Summary for Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)

Runoff $=12.89$ cfs @ 12.17 hrs, Volume $=52,247 \mathrm{cf}$, Depth= 4.40"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

| Area (sf) | CN | Description |
| :---: | :---: | :---: |
| 103,691 | 98 | Impervious Areas |
| 38,867 | 80 | >75\% Grass cover, Good, HSG D |
| 142,558 | 93 | Weighted Average |
| 38,867 | 80 | 27.26\% Pervious Area |
| 103,691 | 98 | 72.74\% Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

## Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)

Runoff $=\quad 7.33$ cfs @ 12.17 hrs, Volume $=\quad 29,650 \mathrm{cf}$, Depth= 4.38"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

|  | Area (sf) | CN |
| ---: | ---: | :--- | Description $\quad$| * | 58,221 | 98 |
| :--- | ---: | :--- |
| Impervious Areas |  |  |
| 23,069 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 81,290 | 93 | Weighted Average |
| 23,069 | 80 | $28.38 \%$ Pervious Area |
| 58,221 | 98 | $71.62 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ |
| ---: | ---: | | Slope |
| ---: |
| $(\mathrm{ft} / \mathrm{ft})$ | | Velocity |
| ---: |
| $(\mathrm{ft} / \mathrm{sec})$ | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, Direct |
| :--- |

Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)


Summary for Subcatchment P-1D: Proposed Drainage to Municipal System
Runoff $=\quad 4.35$ cfs @ 12.17 hrs, Volume $=16,990 \mathrm{cf}$, Depth= 3.87"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 10-Year Rainfall=5.15"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 23,598 | 98 Im | Impervious Areas <br> $>75 \%$ Grass cover, Good, HSG D |  |  |
|  | 29,081 | 80 > |  |  |  |
|  | 52,679 | 88 W | Weighted Average 55.20\% Pervious Area 44.80\% Impervious Area |  |  |
|  | 29,081 | 805 |  |  |  |
|  | 23,598 | 984 |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 10.0 |  |  |  |  | Direct Entry |

## Subcatchment P-1D: Proposed Drainage to Municipal System



Summary for Link E-1: Existing Drainage to Passaic River
Inflow Area = 327,405 sf, $93.75 \%$ Impervious, Inflow Depth $=4.80$ for 10-Year event
Inflow $=31.57$ cfs @ 12.17 hrs , Volume= $\quad 130,864 \mathrm{cf}$

Primary $=31.57$ cfs @ 12.17 hrs , Volume $=130,864 \mathrm{cf}$, Atten $=0 \%$, Lag= 0.0 min
Primary outflow $=$ Inflow, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$

## Link E-1: Existing Drainage to Passaic River



## Summary for Link P-1: Proposed Drainage to Passaic River

Inflow Area = 327,405 sf, $63.05 \%$ Impervious, Inflow Depth $=4.21$ " for 10-Year event Inflow $=\quad 28.71$ cfs @ 12.17 hrs , Volume= $\quad 115,000 \mathrm{cf}$ Primary $=28.71$ cfs @ 12.17 hrs , Volume $=\quad 115,000 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Primary outflow $=$ Inflow, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Link P-1: Proposed Drainage to Passaic River


Time span=0.00-30.00 hrs, $\mathrm{dt}=0.01 \mathrm{hrs}, 3001$ points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method
SubcatchmentE-1A: Existing Drainage to $\begin{gathered}\text { Runoff Area= }=52,576 \mathrm{sf} \\ \text { Tc }=10.0 \text { min } \quad \text { CN }\end{gathered}$ =80/98 Runoff $=6.11 \mathrm{cfs} 25,152 \mathrm{cf}$
SubcatchmentE-1B: Existing Drainage Runoff Area=141,668 sf $100.00 \%$ Impervious Runoff Depth $=6.10$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=0 / 98$ Runoff=17.15 cfs 72,032 cf

SubcatchmentE-1C: Existing Drainageto Runoff Area=80,227 sf $97.90 \%$ Impervious Runoff Depth $=6.07$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=83 / 98$ Runoff $=9.68 \mathrm{cfs} 40,555 \mathrm{cf}$

SubcatchmentE-1D: Existing Drainage to Runoff Area=52,934 sf $82.35 \%$ Impervious Runoff Depth $=5.75$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=6.15 \mathrm{cfs} 25,348 \mathrm{cf}$

SubcatchmentP-1A: Proposed Drainage to Runoff Area=50,878 sf $41.10 \%$ Impervious Runoff Depth=4.92" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=5.34 \mathrm{cfs} 20,844 \mathrm{cf}$

SubcatchmentP-1B: Proposed Drainage Runoff Area=142,558 sf $72.74 \%$ Impervious Runoff Depth=5.55" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff=16.19 cfs $65,968 \mathrm{cf}$

SubcatchmentP-1C: Proposed Drainage to Runoff Area=81,290 sf 71.62\% Impervious Runoff Depth=5.53" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=9.21 \mathrm{cfs} 37,465 \mathrm{cf}$

SubcatchmentP-1D: Proposed Drainage to Runoff Area=52,679 sf $44.80 \%$ Impervious Runoff Depth $=4.99^{\prime \prime}$ $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=5.58 \mathrm{cfs} 21,909 \mathrm{cf}$

Link E-1: Existing Drainage to Passaic River Inflow=39.08 cfs 163,087 cf Primary=39.08 cfs 163,087 cf

Link P-1: Proposed Drainage to Passaic River
Inflow=36.33 cfs 146,186 cf Primary= 36.33 cfs 146,186 cf


Summary for Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)
Runoff $=\quad 6.11$ cfs @ 12.17 hrs, Volume $=\quad 25,152 \mathrm{cf}$, Depth= $5.74{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 43,147 | 98 | Impervious Areas |
| 9,429 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 52,576 | 95 | Weighted Average |
| 9,429 | 80 | 17.93\% Pervious Area |
| 43,147 | 98 | $82.07 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> (feet) | Slope <br> (ft/ft) | Velocity <br> (ft/sec) | Capacity <br> (cfs) |
| ---: | ---: | ---: | ---: | :--- | Description | Direct Entry, Direct |
| :--- |

Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)


Summary for Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)
Runoff $=17.15$ cfs @ 12.17 hrs, Volume $=\quad 72,032 \mathrm{cf}$, Depth= 6.10"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

|  | Area (sf) | CN | Description |
| :--- | ---: | ---: | :--- |
| * | 141,668 | 98 | Impervious Areas |
| 141,668 | 98 | $100.00 \%$ Impervious Area |  |


| Tc <br> $(\mathrm{min})$ | Length <br> (feet) | Slope <br> (ft/ft) | Velocity <br> (ft/sec) | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

## Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)

Runoff $=\quad 9.68$ cfs @ 12.17 hrs, Volume $=\quad 40,555 \mathrm{cf}$, Depth= 6.07"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 78,544 | 98 Im | Impervious Areas |  |  |
|  | 353 | 96 | Gravel surface, HSG D |  |  |
|  | 1,330 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 80,227 | 98 V | Weighted Average |  |  |
|  | 1,683 | 83 | 2.10\% Pervious Area |  |  |
|  | 78,544 | 989 | 97.90\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | $\begin{array}{r} \text { c } \begin{array}{r} \text { Length } \\ \text { (feet) } \\ \hline \end{array} \\ \hline \end{array}$ | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 10.0 |  |  |  |  | Direct Entry |

Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)


Summary for Subcatchment E-1D: Existing Drainage to Municipal System
Runoff $=\quad 6.15$ cfs @ 12.17 hrs, Volume $=\quad 25,348$ cf, Depth= $5.75^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 43,589 | 98 Im | Impervious Areas |  |  |
|  | 9,345 | $80>$ | >75\% Grass cover, Good, HSG D |  |  |
|  | 52,934 | 95 W | Weighted Average |  |  |
|  | 9,345 | 801 | 17.65\% Pervious Area |  |  |
|  | 43,589 | 988 | 82.35\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 10.0 |  |  |  |  | Direct Entry |

Subcatchment E-1D: Existing Drainage to Municipal System


## Summary for Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)

Runoff $=5.34$ cfs @ 12.17 hrs, Volume= $20,844 \mathrm{cf}$, Depth= 4.92"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 20,909 | 98 | Impervious Areas |
| 29,969 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 50,878 | 87 | Weighted Average |
| 29,969 | 80 | $58.90 \%$ Pervious Area |
| 20,909 | 98 | $41.10 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $($ feet $)$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)


## Summary for Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)

Runoff $=16.19$ cfs @ 12.17 hrs, Volume $=\quad 65,968 \mathrm{cf}$, Depth= $5.55^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"


## Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)

Runoff $=9.21$ cfs @ 12.17 hrs, Volume $=\quad 37,465 \mathrm{cf}$, Depth= $5.53^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

|  | Area (sf) | CN |
| ---: | ---: | :--- | Description $\quad$| * | 58,221 | 98 |
| :--- | ---: | :--- |
| Impervious Areas |  |  |
| 23,069 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 81,290 | 93 | Weighted Average |
| 23,069 | 80 | $28.38 \%$ Pervious Area |
| 58,221 | 98 | $71.62 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ |
| ---: | ---: | | Slope |
| ---: |
| $(\mathrm{ft} / \mathrm{ft})$ | | Velocity |
| ---: |
| $(\mathrm{ft} / \mathrm{sec})$ | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, Direct |
| :--- |

Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)


Summary for Subcatchment P-1D: Proposed Drainage to Municipal System
Runoff $=5.58$ cfs @ 12.17 hrs, Volume= $\quad 21,909 \mathrm{cf}$, Depth= 4.99"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 25-Year Rainfall=6.34"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 23,598 | 98 I | Impervious Areas <br> $>75 \%$ Grass cover, Good, HSG D |  |  |
|  | 29,081 | $80>$ |  |  |  |
|  | 52,679 | 88 | Weighted Average 55.20\% Pervious Area 44.80\% Impervious Area |  |  |
|  | 29,081 | 80 |  |  |  |
|  | 23,598 | 98 |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 10.0 |  |  |  |  | Direct Entry |

## Subcatchment P-1D: Proposed Drainage to Municipal System



Summary for Link E-1: Existing Drainage to Passaic River
Inflow Area = 327,405 sf, $93.75 \%$ Impervious, Inflow Depth $=5.98$ " for $25-$ Year event Inflow $=39.08$ cfs @ 12.17 hrs , Volume= $\quad 163,087 \mathrm{cf}$ Primary $=39.08$ cfs @ 12.17 hrs , Volume $=163,087 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.0 min

Primary outflow $=$ Inflow, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$

## Link E-1: Existing Drainage to Passaic River



## Summary for Link P-1: Proposed Drainage to Passaic River

Inflow Area = 327,405 sf, $63.05 \%$ Impervious, Inflow Depth $=5.36$ " for $25-$ Year event Inflow $=36.33$ cfs @ 12.17 hrs , Volume= $\quad 146,186 \mathrm{cf}$ Primary $=36.33$ cfs @ 12.17 hrs , Volume $=146,186 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.0 min

Primary outflow $=$ Inflow, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Link P-1: Proposed Drainage to Passaic River


Time span=0.00-30.00 hrs, $\mathrm{dt}=0.01 \mathrm{hrs}, 3001$ points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method
SubcatchmentE-1A: Existing Drainage to $\begin{gathered}\text { Runoff Area= }=52,576 \mathrm{sf} \\ \text { Tc }=10.0 \text { min } \quad 8.07 \% \text { Impervious }\end{gathered}$ Runoff Depth $=70 / 98$ Runoff $=8.29 \mathrm{cfs} 34,402 \mathrm{cf}$
SubcatchmentE-1B: Existing Drainage Runoff Area=141,668 sf $100.00 \%$ Impervious Runoff Depth $=8.24$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=0 / 98$ Runoff=22.97 cfs $97,278 \mathrm{cf}$

SubcatchmentE-1C: Existing Drainageto Runoff Area=80,227 sf $97.90 \%$ Impervious Runoff Depth $=8.20$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=83 / 98$ Runoff $=12.98 \mathrm{cfs} 54,836 \mathrm{cf}$

SubcatchmentE-1D: Existing Drainage to Runoff Area=52,934 sf $82.35 \%$ Impervious Runoff Depth=7.86" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=8.35 \mathrm{cfs} 34,663 \mathrm{cf}$

SubcatchmentP-1A: Proposed Drainage to Runoff Area=50,878 sf $41.10 \%$ Impervious Runoff Depth $=6.97$ " $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff=7.50 cfs 29,533 cf

SubcatchmentP-1B: Proposed Drainage Runoff Area=142,558 sf 72.74\% Impervious Runoff Depth=7.65" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=22.14 \mathrm{cfs} 90,881 \mathrm{cf}$

SubcatchmentP-1C: Proposed Drainage to Runoff Area=81,290 sf $71.62 \%$ Impervious Runoff Depth=7.63" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=12.60 \mathrm{cfs} 51,659 \mathrm{cf}$

SubcatchmentP-1D: Proposed Drainage to Runoff Area=52,679 sf $44.80 \%$ Impervious Runoff Depth=7.05" $\mathrm{Tc}=10.0 \mathrm{~min} \mathrm{CN}=80 / 98$ Runoff $=7.81 \mathrm{cfs} 30,929 \mathrm{cf}$

Link E-1: Existing Drainage to Passaic River Inflow=52.59 cfs 221,178 cf Primary=52.59 cfs 221,178 cf

Link P-1: Proposed Drainage to Passaic River
Inflow=50.05 cfs 203,003 cf Primary=50.05 cfs 203,003 cf

> Total Runoff Area $=654,810$ sf Runoff Volume $=424,181$ cf Average Runoff Depth $=7.77$ " $21.60 \%$ Pervious $=141,443 \mathrm{sf} \quad 78.40 \%$ Impervious $=513,367 \mathrm{sf}$

Summary for Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)
Runoff $=8.29$ cfs @ 12.17 hrs, Volume $=34,402 \mathrm{cf}$, Depth= $7.85{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 43,147 | 98 | Impervious Areas |
| 9,429 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 52,576 | 95 | Weighted Average |
| 9,429 | 80 | 17.93\% Pervious Area |
| 43,147 | 98 | $82.07 \%$ Impervious Area |

\(\left.$$
\begin{array}{rrrl}\begin{array}{r}\text { Tc } \\
(\mathrm{min})\end{array} & \begin{array}{r}\text { Length } \\
(\mathrm{feet})\end{array} & \begin{array}{r}\text { Slope } \\
(\mathrm{ft} / \mathrm{ft})\end{array} & \begin{array}{r}\text { Velocity } \\
(\mathrm{ft} / \mathrm{sec})\end{array}\end{array}
$$ \begin{array}{r}Capacity <br>

(\mathrm{cfs})\end{array}\right)\) Description | Direct Entry, Direct |
| :--- |

Subcatchment E-1A: Existing Drainage to Existing Conveyance System (18" Pipe)


Summary for Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)
Runoff $=22.97$ cfs @ 12.17 hrs, Volume= 97,278 cf, Depth= 8.24"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

|  | Area (sf) | CN | Description |
| :--- | ---: | ---: | :--- |
| * | 141,668 | 98 | Impervious Areas |
| 141,668 | 98 | $100.00 \%$ Impervious Area |  |


| Tc | Length <br> $(\mathrm{min})$ | Slope <br> (feet) | Velocity <br> (ft/ft) | Capacity <br> (ft/sec) |
| ---: | ---: | ---: | ---: | :--- |
| 10.0 |  |  | Description |  |
| $(\mathrm{cfs})$ |  |  |  |  |

## Subcatchment E-1B: Existing Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)

Runoff $=12.98$ cfs @ 12.17 hrs, Volume $=\quad 54,836 \mathrm{cf}$, Depth= 8.20"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 78,544 | 98 Im | Impervious Areas |  |  |
|  | 353 | 96 | Gravel surface, HSG D |  |  |
|  | 1,330 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 80,227 | 98 V | Weighted Average |  |  |
|  | 1,683 | 83 | 2.10\% Pervious Area |  |  |
|  | 78,544 | 989 | 97.90\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | $\begin{array}{r} \text { c } \begin{array}{r} \text { Length } \\ \text { (feet) } \\ \hline \end{array} \\ \hline \end{array}$ | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 10.0 |  |  |  |  | Direct Entry |

Subcatchment E-1C: Existing Drainageto Existing Conveyance System (15" Pipe)



Summary for Subcatchment E-1D: Existing Drainage to Municipal System
Runoff $=8.35$ cfs @ 12.17 hrs, Volume $=34,663 \mathrm{cf}$, Depth= 7.86

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| * | 43,589 | 98 Im | Impervious Areas |  |  |
|  | 9,345 | $80>$ | >75\% Grass cover, Good, HSG D |  |  |
|  | 52,934 | 95 W | Weighted Average |  |  |
|  | 9,345 | 801 | 17.65\% Pervious Area |  |  |
|  | 43,589 | 988 | 82.35\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 10.0 |  |  |  |  | Direct Entry |

Subcatchment E-1D: Existing Drainage to Municipal System


## Summary for Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)

Runoff $=7.50$ cfs @ 12.17 hrs, Volume $=\quad 29,533 \mathrm{cf}$, Depth= 6.97"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

|  | Area (sf) | CN |
| ---: | ---: | :--- | Description $\quad$| * | 20,909 | 98 |
| :--- | ---: | :--- |
| Impervious Areas |  |  |
| 29,969 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 50,878 | 87 | Weighted Average |
| 29,969 | 80 | $58.90 \%$ Pervious Area |
| 20,909 | 98 | $41.10 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $($ feet $)$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

Subcatchment P-1A: Proposed Drainage to Existing Conveyance System (18" Pipe)


## Summary for Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)

Runoff $=22.14$ cfs @ 12.17 hrs, Volume $=\quad 90,881 \mathrm{cf}$, Depth= $7.65{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 103,691 | 98 | Impervious Areas |
| 38,867 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 142,558 | 93 | Weighted Average |
| 38,867 | 80 | $27.26 \%$ Pervious Area |
| 103,691 | 98 | $72.74 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $($ feet $)$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

## Subcatchment P-1B: Proposed Drainage to Existing Conveyance System (42" Pipe)



## Summary for Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)

Runoff $=12.60$ cfs @ 12.17 hrs, Volume= $51,659 \mathrm{cf}$, Depth= $7.63^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

|  | Area (sf) | CN |
| ---: | ---: | :--- | Description $\quad$| * | 58,221 | 98 |
| :--- | ---: | :--- |
| Impervious Areas |  |  |
| 23,069 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 81,290 | 93 | Weighted Average |
| 23,069 | 80 | $28.38 \%$ Pervious Area |
| 58,221 | 98 | $71.62 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ |
| ---: | ---: | | Slope |
| ---: |
| $(\mathrm{ft} / \mathrm{ft})$ | | Velocity |
| ---: |
| $(\mathrm{ft} / \mathrm{sec})$ | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, Direct |
| :--- |

Subcatchment P-1C: Proposed Drainage to Existing Conveyance System (15" Pipe)


Summary for Subcatchment P-1D: Proposed Drainage to Municipal System
Runoff $=7.81$ cfs @ 12.17 hrs, Volume= $\quad 30,929 \mathrm{cf}$, Depth= 7.05

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/lmperv., Time Span= 0.00-30.00 hrs, dt= 0.01 hrs NOAA 24-hr D 100-Year Rainfall=8.48"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| * | 23,598 | 98 |
| Impervious Areas |  |  |
| 29,081 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 52,679 | 88 | Weighted Average |
| 29,081 | 80 | $55.20 \%$ Pervious Area |
| 23,598 | 98 | $44.80 \%$ Impervious Area |

\(\left.$$
\begin{array}{rr}\begin{array}{r}\text { Tc } \\
(\mathrm{min})\end{array} & \begin{array}{r}\text { Length } \\
(\mathrm{feet})\end{array}\end{array}
$$ $$
\begin{array}{r}\text { Slope } \\
(\mathrm{ft} / \mathrm{ft})\end{array}
$$ $$
\begin{array}{r}\text { Velocity } \\
(\mathrm{ft} / \mathrm{sec})\end{array}
$$ \begin{array}{r}Capacity <br>

(\mathrm{cfs})\end{array}\right)\) Description | Direct Entry, Direct |
| :--- |

## Subcatchment P-1D: Proposed Drainage to Municipal System



Summary for Link E-1: Existing Drainage to Passaic River

| Inflow Area $=$ | $327,405 \mathrm{sf}$, | $93.75 \%$ Impervious, | Inflow Depth $=8.11 "$ | for $100-$ Year event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $52.59 \mathrm{cfs} @$ | 12.17 hrs , Volume= | $221,178 \mathrm{cf}$ |
| Primary | $=$ | $52.59 \mathrm{cfs} @ 12.17 \mathrm{hrs}$, Volume $=$ | $221,178 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |  |

Primary outflow $=$ Inflow, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$

## Link E-1: Existing Drainage to Passaic River



## Summary for Link P-1: Proposed Drainage to Passaic River

Inflow Area $=\quad 327,405$ sf, 63.05\% Impervious, Inflow Depth $=7.44$ " for 100-Year event Inflow $=50.05 \mathrm{cfs} @ 12.17 \mathrm{hrs}$, Volume= $203,003 \mathrm{cf}$ Primary = 50.05 cfs @ 12.17 hrs , Volume $=\quad 203,003 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.0 min

Primary outflow $=$ Inflow, Time Span= $0.00-30.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$
Link P-1: Proposed Drainage to Passaic River


Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan


Storm Sewer Summary Report

| Line No. | Line ID | Flow rate (cfs) | Line Size <br> (in) | Line shape | Line length (ft) | Invert EL Dn (ft) | Invert EL Up (ft) | Line Slope (\%) | HGL Down (ft) | HGL Up <br> (ft) | Minor loss (ft) | HGL Junct (ft) | Dns Line No. | Junction Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1A-E100 | 6.39 | 18 | Cir | 25.000 | 250.91 | 251.04 | 0.520 | 251.97 | 252.10 | 0.37 | 252.46 | End | Grate |
| 2 | E100-D100 | 4.39 | 18 | Cir | 12.000 | 251.04 | 251.83 | 6.583 | 252.46 | 252.63 | n/a | 252.63 j | 1 | Manhole |
| 3 | D100-R100 | 4.39 | 15 | Cir | 10.000 | 253.95 | 254.05 | 1.000 | 254.71 | 254.90 | 0.36 | 254.90 | 2 | None |
| 4 | R100-R101 | 4.14 | 15 | Cir | 68.000 | 254.05 | 254.73 | 1.000 | 254.90 | 255.55 | n/a | 255.55 j | 3 | None |
| 5 | R101-D101 | 3.86 | 15 | Cir | 24.000 | 254.73 | 254.97 | 1.000 | 255.55 | 255.76 | n/a | 255.76 j | 4 | Manhole |
| 6 | D101-YD01 | 3.87 | 15 | Cir | 30.000 | 254.97 | 255.27 | 1.000 | 255.76 | 256.06 | 0.17 | 256.06 | 5 | Grate |
| 7 | YD01-D102 | 3.70 | 15 | Cir | 64.000 | 255.27 | 255.91 | 1.000 | 256.06 | 256.69 | n/a | 256.69 j | 6 | Manhole |
| 8 | D102-D103 | 1.39 | 15 | Cir | 24.000 | 255.91 | 256.15 | 1.000 | 256.69 | 256.61 | 0.09 | 256.61 | 7 | Grate |
| 9 | D103-D104 | 0.53 | 15 | Cir | 70.000 | 256.15 | 256.85 | 1.000 | 256.61 | 257.13 | n/a | 257.13 j | 8 | Grate |
| 10 | R100-BLDG | 0.30 | 6 | Cir | 5.000 | 254.05 | 254.15 | 2.000 | 254.90* | 254.91* | 0.04 | 254.95 | 3 | None |
| 11 | R101-BLDG | 0.30 | 6 | Cir | 5.000 | 254.73 | 254.83 | 2.000 | 255.55* | 255.57* | 0.04 | 255.60 | 4 | None |
| 12 | D102-D105 | 2.38 | 15 | Cir | 155.000 | 255.91 | 259.79 | 2.503 | 256.69 | 260.41 | n/a | 260.41 j | 7 | Grate |
| 13 | 1B-D200 | 16.30 | 42 | Cir | 125.000 | 248.21 | 248.52 | 0.248 | 249.58 | 249.89 | 0.34 | 250.23 | End | Manhole |
| 14 | D200-D201 | 15.79 | 24 | Cir | 11.000 | 250.25 | 250.36 | 1.000 | 251.48 | 251.79 | n/a | 251.79 | 13 | Manhole |
| 15 | D201-D202 | 14.77 | 18 | Cir | 20.000 | 250.36 | 250.56 | 1.000 | 251.86* | 252.26* | 1.09 | 253.34 | 14 | Manhole |
| 16 | D202-D203 | 10.37 | 18 | Cir | 56.000 | 250.56 | 251.12 | 1.000 | 253.34* | 253.89* | 0.27 | 254.16 | 15 | Combination |
| 17 | D203-D204 | 9.38 | 18 | Cir | 46.000 | 251.12 | 251.58 | 1.000 | 254.16* | 254.52* | 0.44 | 254.96 | 16 | Manhole |
| 18 | D204-D205 | 8.88 | 18 | Cir | 83.000 | 251.58 | 252.41 | 1.000 | 254.96* | 255.56* | 0.20 | 255.75 | 17 | Grate |
| 19 | D205-D206 | 8.56 | 18 | Cir | 70.000 | 252.41 | 253.11 | 1.000 | 255.75* | 256.22* | 0.18 | 256.40 | 18 | Grate |
| 20 | D206-D207 | 7.29 | 18 | Cir | 114.000 | 253.11 | 254.25 | 1.000 | 256.40* | 256.95* | 0.26 | 257.22 | 19 | Manhole |
| 21 | D207-D208 | 4.66 | 15 | Cir | 35.000 | 254.25 | 254.60 | 1.000 | 257.22* | 257.40* | 0.25 | 257.65 | 20 | Grate |
| 22 | D208-D209 | 3.15 | 15 | Cir | 39.000 | 254.60 | 254.99 | 1.000 | 257.65* | 257.75* | 0.08 | 257.82 | 21 | Manhole |
| 23 | D209-D210 | 3.21 | 15 | Cir | 86.000 | 254.99 | 255.85 | 1.000 | 257.82* | 258.04* | 0.11 | 258.14 | 22 | Manhole |
| 24 | D210-D211 | 1.42 | 15 | Cir | 53.000 | 255.85 | 256.38 | 1.000 | 258.14* | 258.17* | 0.02 | 258.19 | 23 | Combination |
| Project File: 2019-10-30_Pipe Sizing.stm |  |  |  |  |  |  |  |  | Number of lines: 138 |  |  | Run Date: 10/31/2019 |  |  |
| NOTES: Return period = 25 Yrs. ; *Surcharged (HGL above crown). ; j - Line contains hyd. jump. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Storm Sewer Summary Report

| Line No. | Line ID | Flow rate (cfs) | Line Size <br> (in) | Line shape | Line length (ft) | Invert EL Dn (ft) | Invert EL Up (ft) | Line Slope (\%) | HGL Down (ft) | HGL Up (ft) | Minor loss (ft) | HGL Junct (ft) | Dns Line No. | Junction Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 25 | D200-R200 | 0.59 | 6 | Cir | 5.000 | 257.36 | 257.41 | 1.000 | 257.80 | 257.85 | 0.16 | 258.01 | 13 | None |
| 26 | R200-R201 | 0.59 | 6 | Cir | 11.000 | 257.41 | 257.52 | 1.000 | 258.01* | 258.13* | 0.14 | 258.28 | 25 | None |
| 27 | R201-R202 | 0.30 | 6 | Cir | 68.000 | 257.52 | 258.20 | 1.000 | 258.28 | 258.52 | 0.08 | 258.60 | 26 | None |
| 28 | R202-BLDG | 0.30 | 6 | Cir | 5.000 | 258.20 | 258.25 | 1.000 | 258.60 | 258.53 | 0.11 | 258.53 | 27 | None |
| 29 | R201-BLDG | 0.30 | 6 | Cir | 5.000 | 257.52 | 257.57 | 1.000 | 258.28* | 258.29* | 0.04 | 258.33 | 26 | None |
| 30 | D201-D212 | 1.18 | 15 | Cir | 48.000 | 256.77 | 257.25 | 1.000 | 257.13 | 257.68 | 0.16 | 257.68 | 14 | Combination |
| 31 | D202-YD09 | 1.12 | 6 | Cir | 22.170 | 257.30 | 257.52 | 0.992 | 257.80* | 258.69* | 0.75 | 259.45 | 15 | Grate |
| 32 | YD09 - YD10 | 0.57 | 6 | Cir | 68.380 | 257.52 | 258.20 | 0.994 | 259.45* | 260.15* | 0.20 | 260.35 | 31 | Grate |
| 33 | YD10-BLDG | 0.30 | 6 | Cir | 5.000 | 258.20 | 258.25 | 1.000 | 260.35* | 260.36* | 0.04 | 260.40 | 32 | None |
| 34 | YD09-BLDG | 0.30 | 6 | Cir | 5.000 | 257.57 | 257.62 | 1.000 | 259.45* | 259.46* | 0.04 | 259.50 | 31 | None |
| 35 | D202-YD03 | 3.83 | 15 | Cir | 84.000 | 257.17 | 258.01 | 1.000 | 257.86 | 258.80 | n/a | 258.80 | 15 | Grate |
| 36 | YD03-YD04 | 3.02 | 15 | Cir | 68.000 | 258.01 | 258.69 | 1.000 | 258.80 | 259.39 | n/a | 259.39 j | 35 | Grate |
| 37 | YD04-D213 | 1.91 | 15 | Cir | 30.000 | 258.69 | 258.99 | 1.000 | 259.39 | 259.54 | n/a | 259.54 j | 36 | Manhole |
| 38 | D213-YD05 | 0.23 | 6 | Cir | 16.000 | 258.99 | 259.15 | 1.000 | 259.54 | 259.56 | 0.01 | 259.57 | 37 | Grate |
| 39 | YD05-YD06 | 0.12 | 6 | Cir | 26.000 | 259.15 | 259.41 | 1.000 | 259.57 | 259.58 | n/a | 259.58 j | 38 | Grate |
| 40 | YD03-BLDG | 0.30 | 6 | Cir | 35.000 | 258.01 | 258.36 | 1.000 | 258.80 | 258.86 | 0.04 | 258.90 | 35 | None |
| 41 | YD03-BLDG | 0.30 | 6 | Cir | 11.000 | 258.01 | 258.12 | 1.000 | 258.80* | 258.83* | 0.02 | 258.85 | 35 | None |
| 42 | YD03-BLDG | 0.30 | 6 | Cir | 5.000 | 258.12 | 258.17 | 1.000 | 258.85* | 258.86* | 0.04 | 258.90 | 41 | None |
| 43 | YD04 - BLDG | 0.36 | 6 | Cir | 35.000 | 258.69 | 259.04 | 1.000 | 259.39 | 259.52 | 0.05 | 259.58 | 36 | None |
| 44 | YD04 - BLDG | 0.36 | 6 | Cir | 11.000 | 258.69 | 258.80 | 1.000 | 259.39* | 259.43* | 0.02 | 259.46 | 36 | None |
| 45 | YD04 - BLDG | 0.36 | 6 | Cir | 5.000 | 258.80 | 258.85 | 1.000 | 259.46* | 259.48* | 0.05 | 259.53 | 44 | None |
| 46 | D213-R203 | 1.69 | 15 | Cir | 24.000 | 260.08 | 260.32 | 1.000 | 260.52 | 260.84 | n/a | 260.84 | 37 | None |
| 47 | R203-YD07 | 1.46 | 15 | Cir | 24.000 | 260.32 | 260.56 | 1.000 | 260.84 | 261.04 | n/a | 261.04 j | 46 | Grate |
| 48 | YD07-YD08 | 1.36 | 15 | Cir | 21.000 | 260.56 | 260.77 | 1.000 | 261.04 | 261.23 | n/a | 261.23 j | 47 | Grate |
| Project File: 2019-10-30_Pipe Sizing.stm |  |  |  |  |  |  |  |  | Number of lines: 138 |  |  | Run Date: 10/31/2019 |  |  |
| NOTES: Return period = 25 Yrs. ; *Surcharged (HGL above crown). ; j - Line contains hyd. jump. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Storm Sewer Summary Report

| Line No. | Line ID | Flow rate (cfs) | Line Size <br> (in) | Line shape | Line length (ft) | Invert EL Dn (ft) | Invert EL Up (ft) | Line Slope (\%) | HGL Down (ft) | HGL Up <br> (ft) | Minor loss (ft) | HGL Junct (ft) | Dns Line No. | Junction Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 49 | YD08-D214 | 1.20 | 15 | Cir | 16.000 | 260.77 | 260.93 | 1.000 | 261.23 | 261.36 | n/a | 261.36 j | 48 | Combination |
| 50 | R203-BLDG | 0.24 | 6 | Cir | 6.000 | 260.32 | 260.38 | 1.000 | 260.84 | 260.84 | 0.03 | 260.87 | 46 | None |
| 51 | D204-D215 | 0.90 | 15 | Cir | 21.000 | 251.58 | 251.79 | 1.000 | 254.96* | 254.97* | 0.00 | 254.97 | 17 | Grate |
| 52 | D215-D216 | 0.41 | 15 | Cir | 70.000 | 251.79 | 252.49 | 1.000 | 254.97* | 254.97* | 0.00 | 254.98 | 51 | Grate |
| 53 | D207-D217 | 2.68 | 15 | Cir | 43.000 | 261.50 | 261.93 | 1.000 | 262.06 | 262.59 | n/a | 262.59 | 20 | Combination |
| 54 | D217-D218 | 1.57 | 15 | Cir | 60.000 | 261.93 | 262.53 | 1.000 | 262.59 | 263.03 | n/a | 263.03 j | 53 | Manhole |
| 55 | D218-R204 | 1.37 | 12 | Cir | 13.000 | 264.60 | 264.73 | 1.000 | 265.03 | 265.22 | n/a | 265.22 | 54 | None |
| 56 | R204-R205 | 1.14 | 12 | Cir | 42.000 | 264.73 | 265.15 | 1.000 | 265.22 | 265.60 | n/a | 265.60 j | 55 | None |
| 57 | R205-R206 | 0.91 | 12 | Cir | 42.000 | 265.15 | 265.57 | 1.000 | 265.60 | 265.97 | n/a | 265.97 j | 56 | None |
| 58 | R206-BLDG | 0.24 | 6 | Cir | 10.000 | 265.57 | 265.67 | 1.000 | 265.97 | 265.92 | n/a | 265.92 | 57 | None |
| 59 | R204-BLDG | 0.24 | 6 | Cir | 10.000 | 264.73 | 264.83 | 1.000 | 265.22 | 265.08 | n/a | 265.08 | 55 | None |
| 60 | R205-BLDG | 0.24 | 6 | Cir | 10.000 | 265.15 | 265.25 | 1.000 | 265.60 | 265.50 | n/a | 265.50 | 56 | None |
| 61 | D217-R208 | 0.22 | 6 | Cir | 69.000 | 264.60 | 265.29 | 1.000 | 264.82 | 265.53 | n/a | 265.53 | 54 | None |
| 62 | R208-YD11 | 0.23 | 6 | Cir | 16.000 | 265.29 | 265.45 | 1.000 | 265.53 | 265.69 | 0.05 | 265.69 | 61 | Grate |
| 63 | YD11-YD12 | 0.17 | 6 | Cir | 32.000 | 265.45 | 265.77 | 1.000 | 265.69 | 265.98 | n/a | 265.98 j | 62 | Grate |
| 64 | YD12-YD13 | 0.11 | 6 | Cir | 32.000 | 265.77 | 266.09 | 1.000 | 265.98 | 266.26 | n/a | 266.26 j | 63 | Grate |
| 65 | D210-R209 | 1.84 | 12 | Cir | 29.000 | 258.75 | 259.04 | 1.000 | 259.26 | 259.62 | n/a | 259.62 | 23 | None |
| 66 | R209-R210 | 0.96 | 12 | Cir | 68.000 | 259.04 | 259.72 | 1.000 | 259.62 | 260.13 | n/a | 260.13 j | 65 | None |
| 67 | R209-BLDG | 0.30 | 6 | Cir | 37.000 | 259.04 | 259.41 | 1.000 | 259.62 | 259.73 | 0.08 | 259.81 | 65 | None |
| 68 | R209-YD14 | 0.61 | 6 | Cir | 11.000 | 259.04 | 259.15 | 1.000 | 259.62* | 259.75* | 0.08 | 259.82 | 65 | Grate |
| 69 | YD14 - BLDG | 0.30 | 6 | Cir | 10.000 | 259.15 | 259.25 | 1.000 | 259.82* | 259.85* | 0.04 | 259.89 | 68 | None |
| 70 | R210-BLDG | 0.30 | 6 | Cir | 37.000 | 259.72 | 260.09 | 1.000 | 260.13 | 260.37 | n/a | 260.37 j | 66 | None |
| 71 | R210-YD15 | 0.66 | 6 | Cir | 11.000 | 259.72 | 259.83 | 1.000 | 260.22 | 260.33 | 0.09 | 260.42 | 66 | Grate |
| 72 | YD15 - BLDG | 0.30 | 6 | Cir | 10.000 | 259.83 | 259.93 | 1.000 | 260.42 | 260.43 | 0.04 | 260.47 | 71 | None |
| Project File: 2019-10-30_Pipe Sizing.stm |  |  |  |  |  |  |  |  | Number of lines: 138 |  |  | Run Date: 10/31/2019 |  |  |
| NOTES: Return period = 25 Yrs. ; *Surcharged (HGL above crown). ; j - Line contains hyd. jump. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Storm Sewer Summary Report

| Line No. | Line ID | Flow rate (cfs) | Line Size <br> (in) | Line shape | Line length (ft) | Invert EL Dn (ft) | Invert EL Up (ft) | Line Slope (\%) | HGL Down (ft) | HGL Up <br> (ft) | Minor loss (ft) | HGL Junct (ft) | Dns Line No. | Junction Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 73 | R206-R207 | 0.67 | 12 | Cir | 30.000 | 265.57 | 265.87 | 1.000 | 265.97 | 266.21 | n/a | 266.21 j | 57 | None |
| 74 | R207-D219 | 0.68 | 12 | Cir | 25.000 | 265.87 | 266.12 | 1.000 | 266.21 | 266.46 | n/a | 266.46 | 73 | Grate |
| 75 | 1C-D300 | 9.63 | 15 | Cir | 50.000 | 244.75 | 245.00 | 0.500 | 246.00* | 247.11* | 1.12 | 248.23 | End | Grate |
| 76 | D300-D301 | 9.31 | 15 | Cir | 24.000 | 248.50 | 249.22 | 3.000 | 249.37 | 250.39 | n/a | 250.39 | 75 | Manhole |
| 77 | D301-D302 | 8.15 | 15 | Cir | 69.000 | 249.22 | 250.60 | 2.000 | 250.39 | 251.72 | n/a | 251.72 j | 76 | Manhole |
| 78 | D302-D303 | 7.36 | 15 | Cir | 30.000 | 250.75 | 251.05 | 1.000 | 252.00* | 252.39* | 0.80 | 253.19 | 77 | Grate |
| 79 | D303-D304 | 4.08 | 15 | Cir | 114.000 | 251.05 | 252.19 | 1.000 | 253.19* | 253.65* | 0.26 | 253.91 | 78 | Grate |
| 80 | D304-D305 | 3.24 | 15 | Cir | 66.000 | 252.19 | 252.85 | 1.000 | 253.91 | 254.06 | 0.06 | 254.12 | 79 | Grate |
| 81 | D305-D306 | 2.44 | 15 | Cir | 62.000 | 252.85 | 253.47 | 1.000 | 254.12 | 254.16 | 0.10 | 254.26 | 80 | Grate |
| 82 | D306-D307 | 1.89 | 15 | Cir | 70.000 | 253.47 | 254.17 | 1.000 | 254.26 | 254.72 | n/a | 254.72 j | 81 | Grate |
| 83 | D307-D308 | 1.40 | 15 | Cir | 62.000 | 254.17 | 254.79 | 1.000 | 254.72 | 255.26 | n/a | 255.26 j | 82 | Grate |
| 84 | D308-D309 | 1.12 | 15 | Cir | 70.000 | 254.79 | 255.49 | 1.000 | 255.26 | 255.91 | n/a | 255.91 j | 83 | Grate |
| 85 | D301-R300 | 0.60 | 8 | Cir | 10.000 | 254.17 | 254.27 | 1.000 | 254.50 | 254.63 | n/a | 254.63 | 76 | None |
| 86 | R300-R301 | 0.30 | 8 | Cir | 21.000 | 254.27 | 254.48 | 1.000 | 254.63 | 254.73 | n/a | 254.73 j | 85 | None |
| 87 | R301-BLDG | 0.30 | 6 | Cir | 5.000 | 254.48 | 254.53 | 1.000 | 254.74 | 254.81 | 0.11 | 254.81 | 86 | None |
| 88 | R300-R302 | 0.30 | 8 | Cir | 48.000 | 254.27 | 254.75 | 1.000 | 254.63 | 255.00 | n/a | 255.00 j | 85 | None |
| 89 | R302-BLDG | 0.30 | 6 | Cir | 5.000 | 254.75 | 254.80 | 1.000 | 255.01 | 255.08 | 0.11 | 255.08 | 88 | None |
| 90 | D301-YD16 | 0.69 | 8 | Cir | 32.000 | 252.22 | 252.54 | 1.000 | 252.58 | 252.93 | 0.25 | 252.93 | 76 | Grate |
| 91 | YD16-YD17 | 0.35 | 8 | Cir | 68.000 | 252.54 | 253.22 | 1.000 | 252.93 | 253.50 | n/a | 253.50 j | 90 | Grate |
| 92 | YD17-BLDG | 0.30 | 6 | Cir | 7.000 | 253.22 | 253.29 | 1.000 | 253.50 | 253.57 | 0.11 | 253.57 | 91 | None |
| 93 | YD16 - BLDG | 0.30 | 6 | Cir | 7.000 | 252.54 | 252.61 | 1.000 | 252.93 | 252.89 | 0.11 | 252.89 | 90 | None |
| 94 | D302-D310 | 0.89 | 15 | Cir | 42.000 | 254.58 | 255.00 | 1.000 | 254.89 | 255.37 | n/a | 255.37 | 77 | Combination |
| 95 | D303-R303 | 2.78 | 15 | Cir | 75.000 | 253.85 | 254.60 | 1.000 | 254.42 | 255.27 | n/a | 255.27 | 78 | None |
| 96 | R303-R304 | 1.71 | 15 | Cir | 69.000 | 254.60 | 255.29 | 1.000 | 255.27 | 255.81 | n/a | 255.81 j | 95 | None |
| Project File: 2019-10-30_Pipe Sizing.stm |  |  |  |  |  |  |  |  | Number of lines: 138 |  |  | Run Date: 10/31/2019 |  |  |
| NOTES: Return period = 25 Yrs. ; *Surcharged (HGL above crown). ; j - Line contains hyd. jump. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Storm Sewer Summary Report

| Line No. | Line ID | Flow rate (cfs) | Line Size <br> (in) | Line shape | Line length (ft) | Invert EL Dn (ft) | Invert EL Up (ft) | Line Slope (\%) | HGL Down (ft) | HGL Up <br> (ft) | Minor loss (ft) | HGL Junct (ft) | Dns Line No. | Junction Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 97 | R304-R305 | 0.87 | 15 | Cir | 26.000 | 255.29 | 255.55 | 1.000 | 255.81 | 255.91 | n/a | 255.91 j | 96 | None |
| 98 | R303-BLDG | 0.30 | 6 | Cir | 37.000 | 254.60 | 254.97 | 1.000 | 255.27 | 255.37 | 0.05 | 255.42 | 95 | None |
| 99 | R303-YD18 | 0.82 | 6 | Cir | 11.000 | 254.60 | 254.71 | 1.000 | 255.27* | 255.50* | 0.14 | 255.64 | 95 | Grate |
| 100 | YD18-BLDG | 0.30 | 6 | Cir | 10.000 | 254.71 | 254.81 | 1.000 | 255.64* | 255.67* | 0.04 | 255.71 | 99 | None |
| 101 | R304-BLDG | 0.30 | 6 | Cir | 37.000 | 255.29 | 255.66 | 1.000 | 255.81 | 255.94 | n/a | 256.05 j | 96 | None |
| 102 | R304-YD19 | 0.57 | 6 | Cir | 11.000 | 255.29 | 255.40 | 1.000 | 255.81 | 255.90 | 0.07 | 255.96 | 96 | Grate |
| 103 | YD19 - BLDG | 0.30 | 6 | Cir | 10.000 | 255.40 | 255.50 | 1.000 | 255.96 | 255.99 | 0.04 | 256.03 | 102 | None |
| 104 | R305-YD20 | 0.27 | 6 | Cir | 37.000 | 255.55 | 255.92 | 1.000 | 255.91 | 256.18 | n/a | 256.18 j | 97 | Grate |
| 105 | YD20-YD21 | 0.22 | 6 | Cir | 25.000 | 255.92 | 256.17 | 1.000 | 256.18 | 256.40 | n/a | 256.40 j | 104 | Grate |
| 106 | R305-YD22 | 0.60 | 6 | Cir | 21.000 | 255.55 | 255.76 | 1.000 | 256.01 | 256.22 | 0.08 | 256.30 | 97 | Grate |
| 107 | YD22-YD23 | 0.27 | 6 | Cir | 25.000 | 255.76 | 256.01 | 1.000 | 256.30 | 256.34 | 0.03 | 256.37 | 106 | Grate |
| 108 | YD23-YD24 | 0.11 | 6 | Cir | 18.000 | 256.01 | 256.19 | 1.000 | 256.37 | 256.36 | 0.06 | 256.36 | 107 | Grate |
| 109 | 1D-D400 | 3.96 | 15 | Cir | 100.000 | 236.31 | 243.81 | 7.500 | 236.71 | 244.61 | 0.52 | 244.61 | End | Grate |
| 110 | D400-D401 | 2.56 | 15 | Cir | 10.000 | 244.67 | 244.97 | 3.000 | 245.08 | 245.61 | 0.25 | 245.61 | 109 | Manhole |
| 111 | D401-R400 | 1.88 | 15 | Cir | 135.000 | 248.00 | 252.05 | 3.000 | 248.35 | 252.60 | n/a | 252.60 | 110 | None |
| 112 | R400-R401 | 1.17 | 15 | Cir | 132.000 | 252.05 | 256.01 | 3.000 | 252.60 | 256.44 | n/a | 256.44 j | 111 | None |
| 113 | D401-R402 | 0.75 | 10 | Cir | 18.000 | 248.00 | 251.60 | 20.000 | 248.16 | 251.98 | 0.15 | 251.98 | 110 | None |
| 114 | R402-R403 | 0.30 | 6 | Cir | 31.000 | 251.60 | 251.91 | 1.000 | 251.98 | 252.19 | n/a | 252.19 j | 113 | None |
| 115 | R403-BLDG | 0.30 | 6 | Cir | 3.000 | 251.91 | 251.94 | 1.000 | 252.19 | 252.22 | 0.11 | 252.22 | 114 | None |
| 116 | R402-R404 | 0.46 | 6 | Cir | 37.000 | 251.60 | 251.97 | 1.000 | 251.98 | 252.31 | 0.16 | 252.31 | 113 | None |
| 117 | R404-R405 | 0.16 | 6 | Cir | 17.000 | 251.97 | 252.14 | 1.000 | 252.31 | 252.34 | n/a | 252.34 j | 116 | None |
| 118 | R405-YD25 | 0.16 | 6 | Cir | 11.000 | 252.14 | 252.25 | 1.000 | 252.34 | 252.45 | 0.08 | 252.45 | 117 | Grate |
| 119 | YD25-YD26 | 0.11 | 6 | Cir | 25.000 | 252.25 | 252.50 | 1.000 | 252.45 | 252.66 | n/a | 252.66 j | 118 | Grate |
| 120 | R404-BLDG | 0.30 | 6 | Cir | 7.000 | 251.97 | 252.04 | 1.000 | 252.31 | 252.32 | 0.11 | 252.32 | 116 | None |
| Project File: 2019-10-30_Pipe Sizing.stm |  |  |  |  |  |  |  |  | Number of lines: 138 |  |  | Run Date: 10/31/2019 |  |  |
| NOTES: Return period = 25 Yrs. ; *Surcharged (HGL above crown). ; j - Line contains hyd. jump. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Storm Sewer Summary Report

| Line No. | Line ID | Flow rate (cfs) | Line Size <br> (in) | Line shape | Line length (ft) | Invert EL Dn (ft) | Invert EL Up (ft) | Line Slope (\%) | HGL Down (ft) | HGL Up <br> (ft) | Minor loss (ft) | HGL Junct (ft) | Dns Line No. | Junction Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 121 | R400-R406 | 0.78 | 10 | Cir | 15.000 | 252.05 | 252.20 | 1.000 | 252.60 | 252.59 | n/a | 252.59 j | 111 | None |
| 122 | R406-R407 | 0.30 | 6 | Cir | 34.000 | 252.20 | 252.54 | 1.000 | 252.59 | 252.82 | n/a | 252.82 j | 121 | None |
| 123 | R407-BLDG | 0.30 | 6 | Cir | 10.000 | 252.54 | 252.64 | 1.000 | 252.82 | 252.92 | 0.11 | 252.92 | 122 | None |
| 124 | R406-R408 | 0.49 | 6 | Cir | 34.000 | 252.20 | 252.54 | 1.000 | 252.59 | 252.90 | n/a | 253.06 j | 121 | None |
| 125 | R408-R409 | 0.19 | 6 | Cir | 14.000 | 252.54 | 252.68 | 1.000 | 253.06 | 253.07 | 0.02 | 253.09 | 124 | None |
| 126 | R409-YD27 | 0.19 | 6 | Cir | 15.000 | 252.68 | 252.83 | 1.000 | 253.09 | 253.05 | 0.09 | 253.05 | 125 | Grate |
| 127 | YD27-YD28 | 0.13 | 6 | Cir | 25.000 | 252.83 | 253.08 | 1.000 | 253.05 | 253.26 | n/a | 253.26 j | 126 | Grate |
| 128 | R408-BLDG | 0.30 | 6 | Cir | 10.000 | 252.54 | 252.64 | 1.000 | 253.06 | 253.08 | 0.04 | 253.12 | 124 | None |
| 129 | R401-R410 | 1.17 | 10 | Cir | 15.000 | 256.01 | 256.16 | 1.000 | 256.44 | 256.64 | 0.20 | 256.64 | 112 | None |
| 130 | R410-R411 | 0.30 | 6 | Cir | 34.000 | 256.16 | 256.50 | 1.000 | 256.64 | 256.78 | n/a | 256.78 j | 129 | None |
| 131 | R411-BLDG | 0.30 | 6 | Cir | 10.000 | 256.50 | 256.60 | 1.000 | 256.78 | 256.88 | 0.11 | 256.88 | 130 | None |
| 132 | R410-R412 | 0.89 | 6 | Cir | 34.000 | 256.16 | 256.50 | 1.000 | 256.66* | 257.51* | 0.32 | 257.83 | 129 | None |
| 133 | R412-R413 | 0.60 | 6 | Cir | 14.000 | 256.50 | 256.64 | 1.000 | 257.83* | 257.99* | 0.11 | 258.10 | 132 | None |
| 134 | R413-YD29 | 0.60 | 6 | Cir | 15.000 | 256.64 | 256.79 | 1.000 | 258.10* | 258.27* | 0.16 | 258.43 | 133 | Grate |
| 135 | YD29 - YD30 | 0.50 | 6 | Cir | 25.000 | 256.79 | 257.04 | 1.000 | 258.43* | 258.63* | 0.05 | 258.68 | 134 | Grate |
| 136 | YD30 - YD31 | 0.35 | 6 | Cir | 32.000 | 257.04 | 257.36 | 1.000 | 258.68* | 258.81* | 0.03 | 258.84 | 135 | Grate |
| 137 | YD31-YD32 | 0.21 | 6 | Cir | 37.000 | 257.36 | 257.73 | 1.000 | 258.84* | 258.89* | 0.02 | 258.90 | 136 | Grate |
| 138 | R412-BLDG | 0.30 | 6 | Cir | 10.000 | 256.50 | 256.60 | 1.000 | 257.83* | 257.86* | 0.04 | 257.89 | 132 | None |
| Project File: 2019-10-30_Pipe Sizing.stm |  |  |  |  |  |  |  |  | Number of lines: 138 |  |  | Run Date: 10/31/2019 |  |  |
| NOTES: Return period = 25 Yrs. ; *Surcharged (HGL above crown). ; j - Line contains hyd. jump. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |



Line Profile (Line 2) - E100 - D100


## Line Profile (Line 3) - D100 - R100




|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 4 | 4.14 | 254.05 | 254.73 | 0.85 | 0.82 | 0.82 | 254.90 | 255.55 j | 255.55 | 4.66 | 4.83 | 5.80 | 5.12 |





|  | $\begin{aligned} & \text { Q } \\ & \text { (cfs) } \end{aligned}$ | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn <br> (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 7 | 3.70 | 255.27 | 255.91 | 0.79 | 0.78 | 0.78 | 256.06 | 256.69 j | 256.69 | 4.49 | 4.62 | 4.48 | 3.34 |

Line Profile (Line 8) - D102 - D103



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 9 | 0.53 | 256.15 | 256.85 | 0.46 | 0.28 | 0.28 | 256.61 | 257.13 j | 257.13 | 1.27 | 2.53 | 2.45 | 1.75 |




## Line Profile (Line 12) - D102 - D105



|  |  | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# | $\begin{aligned} & Q \\ & \text { (cfs) } \end{aligned}$ | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn <br> (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 12 | 2.38 | 255.91 | 259.79 | 0.78 | 0.62 | 0.62 | 256.69 | 260.41 j | 260.41 | 2.97 | 3.95 | 3.34 | 2.88 |

Line Profile (Line 13) - 1B - D200


|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 13 | 16.30 | 248.21 | 248.52 | 3.50 | 3.22 | 3.26 | 251.71 | 251.74 | 251.78 | 1.69 | 1.76 | 8.29 | 8.20 |

Line Profile (Line 14) - D200 - D201


Line Profile (Line 15) - D201 -D202


| Line \# | Q (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 15 | 14.77 | 250.36 | 250.56 | 1.50 | 1.50 | 2.78 | 251.86 | 252.26 | 253.34 | 8.36 | 8.36 | 8.77 | 8.11 |
| Project File: |  |  |  |  |  |  |  | No. Lines: 138 |  |  | Run Date: | : 10/31/2019 |  |



|  | Q (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn <br> (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 16 | 10.37 | 250.56 | 251.12 | 1.50 | 1.50 | 3.04 | 253.34 | 253.89 | 254.16 | 5.87 | 5.87 | 8.11 | 6.88 |



| Line \# | Q (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 17 | 9.38 | 251.12 | 251.58 | 1.50 | 1.50 | 3.38 | 254.16 | 254.52 | 254.96 | 5.31 | 5.31 | 6.88 | 5.72 |



|  | Q (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn <br> (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 18 | 8.88 | 251.58 | 252.41 | 1.50 | 1.50 | 3.34 | 254.96 | 255.56 | 255.75 | 5.03 | 5.03 | 5.72 | 7.59 |



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn $(\mathrm{ft} / \mathrm{s})$ | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 19 | 8.56 | 252.41 | 253.11 | 1.50 | 1.50 | 3.29 | 255.75 | 256.22 | 256.40 | 4.85 | 4.84 | 7.59 | 6.47 |

Line Profile (Line 20) - D206-D207


|  | $\begin{aligned} & \text { Q } \\ & \text { (cfs) } \end{aligned}$ | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 20 | 7.29 | 253.11 | 254.25 | 1.50 | 1.50 | 2.97 | 256.40 | 256.95 | 257.22 | 4.13 | 4.13 | 6.47 | 8.75 |



| Line \# | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 21 | 4.66 | 254.25 | 254.60 | 1.25 | 1.25 | 3.05 | 257.22 | 257.40 | 257.65 | 3.80 | 3.80 | 9.00 | 8.57 |
| Project File: |  |  |  |  |  |  |  | No. Lines: 138 |  |  | Run Date: | 10/31/2019 |  |



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 22 | 3.15 | 254.60 | 254.99 | 1.25 | 1.25 | 2.83 | 257.65 | 257.75 | 257.82 | 2.57 | 2.57 | 8.57 | 8.95 |



| Line \# | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn <br> (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 23 | 3.21 | 254.99 | 255.85 | 1.25 | 1.25 | 2.29 | 257.82 | 258.04 | 258.14 | 2.62 | 2.62 | 8.95 | 4.65 |



|  | $\begin{aligned} & \text { Q } \\ & \text { (cfs) } \end{aligned}$ | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 24 | 1.42 | 255.85 | 256.38 | 1.25 | 1.25 | 1.81 | 258.14 | 258.17 | 258.19 | 1.15 | 1.15 | 4.65 | 1.77 |





|  |  | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# | $\begin{aligned} & Q \\ & \text { (cfs) } \end{aligned}$ | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 27 | 0.30 | 257.52 | 258.20 | 0.50 | 0.32 | 0.40 | 258.28 | 258.52 | 258.60 | 1.54 | 2.31 | 2.98 | 2.30 |





|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 30 | 1.18 | 256.77 | 257.25 | 0.36 | 0.43 | 0.43 | 257.13 | 257.68 | 257.68 | 4.01 | 3.18 | 2.61 | 1.75 |




|  |  | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn <br> (ft/s) | Up <br> (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 32 | 0.57 | 257.52 | 258.20 | 0.50 | 0.50 | 2.15 | 259.45 | 260.15 | 260.35 | 2.90 | 2.89 | 1.73 | 1.05 |

Line Profile (Line 33) - YD10 - BLDG




|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn $(\mathrm{ft})$ | Up <br> (ft) | Jnct <br> (ft) | Dn <br> (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 35 | 3.83 | 257.17 | 258.01 | 0.69 | 0.79 | 0.79 | 257.86 | 258.80 | 258.80 | 5.48 | 4.68 | 1.75 | 2.99 |



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 36 | 3.02 | 258.01 | 258.69 | 0.79 | 0.70 | 0.70 | 258.80 | 259.39 j | 259.39 | 3.69 | 4.28 | 2.99 | 2.31 |



Line Profile (Line 38) - D213 - YD05



## Line Profile (Line 40) - YD03 - BLDG




Line Profile (Line 42) - YD03 - BLDG




Line Profile (Line 45) - YD04 - BLDG



## Line Profile (Line 47) - R203 - YD07




Line Profile (Line 49) - YD08 - D214





|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 52 | 0.41 | 251.79 | 252.49 | 1.25 | 1.25 | 2.49 | 254.97 | 254.97 | 254.98 | 0.34 | 0.34 | 4.81 | 4.11 |



|  | $\begin{aligned} & \text { Q } \\ & \text { (cfs) } \end{aligned}$ | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 53 | 2.68 | 261.50 | 261.93 | 0.56 | 0.66 | 0.66 | 262.06 | 262.59 | 262.59 | 5.02 | 4.11 | 1.75 | 2.82 |



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 54 | 1.57 | 261.93 | 262.53 | 0.66 | 0.50 | 0.50 | 262.59 | 263.03 j | 263.03 | 2.41 | 3.46 | 2.82 | 3.82 |




## Line Profile (Line 57) - R205-R206




Line Profile (Line 60) - R205 - BLDG



|  |  | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# | $\begin{aligned} & Q \\ & \text { (cfs) } \end{aligned}$ | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 61 | 0.22 | 264.60 | 265.29 | 0.22 | 0.24 | 0.24 | 264.82 | 265.53 | 265.53 | 2.70 | 2.44 | 2.50 | 4.16 |




Line Profile (Line 64) - YD12 - YD13




|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 66 | 0.96 | 259.04 | 259.72 | 0.58 | 0.41 | 0.41 | 259.62 | 260.13 j | 260.13 | 2.04 | 3.16 | 2.21 | 1.38 |




Line Profile (Line 69) - YD14 - BLDG



Line Profile (Line 71) - R210 - YD15




Line Profile (Line 74) - R207 - D219





|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 77 | 8.15 | 249.22 | 250.60 | 1.16 | 1.12 | 1.12 | 250.39 | 251.72 j | 251.72 | 6.85 | 7.02 | 6.70 | 6.73 |



|  | $\begin{aligned} & \text { Q } \\ & \text { (cfs) } \end{aligned}$ | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 79 | 4.08 | 251.05 | 252.19 | 1.25 | 1.25 | 1.72 | 253.19 | 253.65 | 253.91 | 3.33 | 3.32 | 4.55 | 2.41 |



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 80 | 3.24 | 252.19 | 252.85 | 1.25 | 1.21 | 1.27 | 253.91 | 254.06 | 254.12 | 2.64 | 2.67 | 2.41 | 1.75 |

Line Profile (Line 81) - D305 - D306


| Line \# | Q (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 81 | 2.44 | 252.85 | 253.47 | 1.25 | 0.69 | 0.79 | 254.12 | 254.16 | 254.26 | 1.99 | 3.51 | 1.75 | 2.78 |



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 82 | 1.89 | 253.47 | 254.17 | 0.79 | 0.55 | 0.55 | 254.26 | 254.72 j | 254.72 | 2.33 | 3.67 | 2.78 | 3.13 |



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 83 | 1.40 | 254.17 | 254.79 | 0.55 | 0.47 | 0.47 | 254.72 | 255.26 j | 255.26 | 2.70 | 3.34 | 3.13 | 3.81 |



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 84 | 1.12 | 254.79 | 255.49 | 0.47 | 0.42 | 0.42 | 255.26 | 255.91 j | 255.91 | 2.68 | 3.13 | 3.81 | 3.11 |






|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 88 | 0.30 | 254.27 | 254.75 | 0.36 | 0.25 | 0.25 | 254.63 | 255.00 j | 255.00 | 1.56 | 2.47 | 4.16 | 3.68 |



## Line Profile (Line 90) - D301 - YD16




|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 91 | 0.35 | 252.54 | 253.22 | 0.39 | 0.28 | 0.28 | 252.93 | 253.50 j | 253.50 | 1.65 | 2.59 | 4.79 | 4.11 |

Line Profile (Line 92) - YD17-BLDG



Line Profile (Line 94) - D302 - D310



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 95 | 2.78 | 253.85 | 254.60 | 0.57 | 0.67 | 0.67 | 254.42 | 255.27 | 255.27 | 5.06 | 4.16 | 1.75 | 3.15 |



|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 96 | 1.71 | 254.60 | 255.29 | 0.67 | 0.52 | 0.52 | 255.27 | 255.81 j | 255.81 | 2.57 | 3.55 | 3.15 | 4.46 |




Line Profile (Line 99) - R303-YD18











## Line Profile (Line 109) - 1D - D400



| Line \# | Q (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 109 | 3.96 | 236.31 | 243.81 | 1.25 | 0.80 | 0.80 | 237.56 | 244.61 j | 244.61 | 3.22 | 4.74 | 2.94 | 2.61 |




|  | Q <br> (cfs) | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# |  | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct (ft) | Dn <br> (ft/s) | Up <br> (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 111 | 1.88 | 248.00 | 252.05 | 0.35 | 0.55 | 0.55 | 248.35 | 252.60 | 252.60 | 6.77 | 3.66 | 1.25 | 2.95 |



|  |  | Invert Elevation |  | Depth of Flow |  |  | Hydraulic Grade Line |  |  | Velocity |  | Cover |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Line \# | $\begin{aligned} & Q \\ & \text { (cfs) } \end{aligned}$ | Dn <br> (ft) | Up <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Hw <br> (ft) | Dn <br> (ft) | Up <br> (ft) | Jnct <br> (ft) | Dn (ft/s) | Up (ft/s) | Dn <br> (ft) | Up <br> (ft) |
| 112 | 1.17 | 252.05 | 256.01 | 0.55 | 0.43 | 0.43 | 252.60 | 256.44 j | 256.44 | 2.27 | 3.17 | 2.95 | 2.84 |






Line Profile (Line 117) - R404-R405


Line Profile (Line 118) - R405-YD25




## Line Profile (Line 121) - R400-R406



## Line Profile (Line 122) - R406-R407



## Line Profile (Line 124) - R406-R408



Line Profile (Line 126) - R409-YD27


Line Profile (Line 127) - YD27 - YD28


## Line Profile (Line 128) - R408 - BLDG













# APPENDIXD Drainage Area Maps 

## INVENTORY

Sheet I of 3: Existing Drainage Area Map
Sheet 2 of 3: Proposed Drainage Area Map
Sheet 3 of 3: Proposed Inlet Area Map





[^0]:    ${ }^{1}$ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).
    Numbers in parenthesis are PF estimates at lower and upper bounds of the $90 \%$ confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is $5 \%$. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.
    Please refer to NOAA Atlas 14 document for more information.

[^1]:    Total Runoff Area $=654,810$ sf Runoff Volume $=245,864$ cf Average Runoff Depth $=4.51$ " $\mathbf{2 1 . 6 0 \%}$ Pervious $=141,443$ sf $78.40 \%$ Impervious $=513,367$ sf

